Minidoka Project, Idaho-Wyoming North Side Pumping Division Extension

HYDROLOGY APPENDIX

July 1985

Copy No. 1

# Minidoka Project, Idaho-Wyoming North Side Pumping Division Extension

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Bureau of Reclamation Boise, Idaho July 1985

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### HYDROLOGY APPENDIX

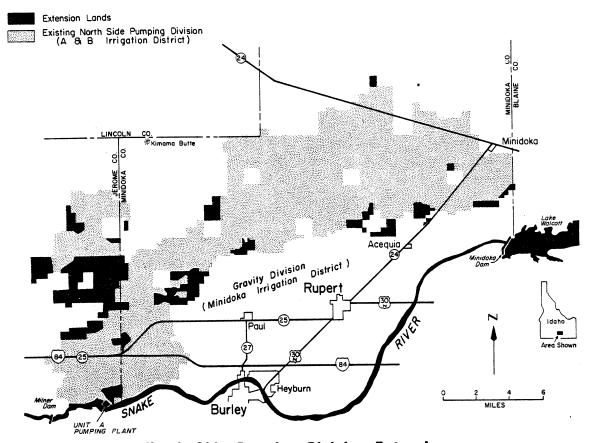
### INTRODUCTION

The Minidoka North Side Pumping Division Extension project study is being conducted under the authority of Public Law 92-199, December 15, 1971. The North Side Pumping Division Extension of the Minidoka Project is located on the southern portion of the Snake River Plain and occupies a slightly elevated belt of land to the north and west of the Gravity Division (part of the Minidoka Project) in Minidoka and Jerome Counties (see North Side Pumping Division Extension map). The surface in the general study area is flat to gently rolling, with smooth benches and some small knobs. The elevation of most of the Extension lands are from 4200 to 4300 feet.

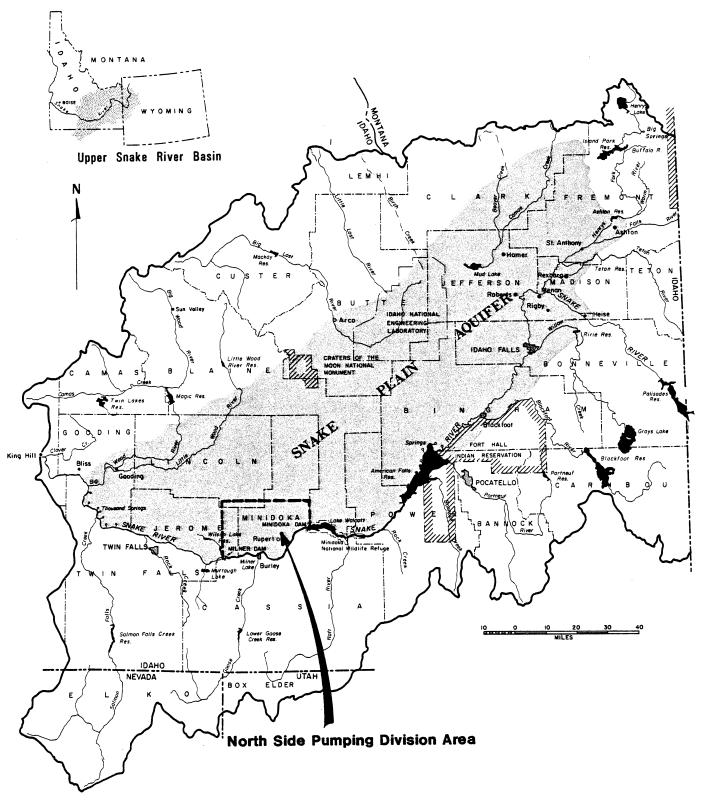
The study area is underlain by the Snake Plain aquifer, one of the most productive ground-water aquifers in the world (see Upper Snake River Basin map). The study area is located on the lower portion of the aquifer, which benefits from recharge resulting from surface water diverted to irrigate thousands of acres of lands upstream. Most of the discharge from the lower portion of the aquifer occurs downstream from the study area in the general area of Thousand Springs; more than 4 million acre-feet is discharged in that reach annually.

The potential Extension includes about 13,410 acres of dry Federal land under Reclamation withdrawal and about 3,750 acres of Bureau of Land Management land. For convenience, both the Reclamation withdrawn lands and the Bureau of Land Management lands are referred to as the Extension land in this study.

The Recommended Plan proposes to provide a full water supply to about 10,210 acres of land scattered throughout the North Side Pumping Division. This includes 9,400 acres of Extension land and 810 acres to receive a



DEC 1983 North Side Pumping Division Extension



**UPPER SNAKE RIVER BASIN** 

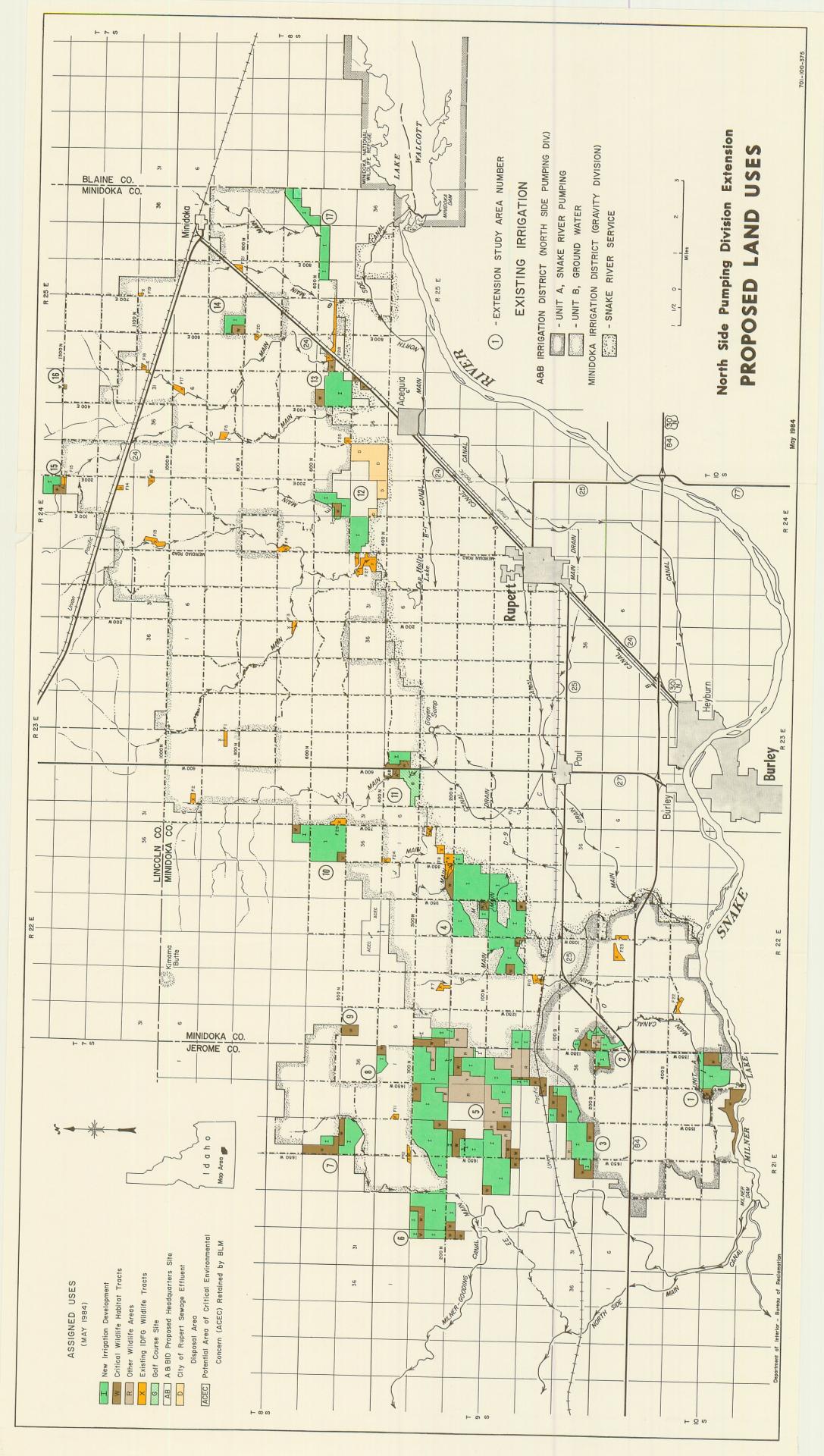
replacement water supply. The lands are located in 14 separate tracts (see Proposed Land Uses map) and range in size from 10 to more than 5,000 acres. Some of this land was considered for development with the rest of the original North Side Pumping Division, but funding limitations prevented its inclusion in the project at that time. Under the Recommended Plan, tracts 1, 2, 3, and 4 will be served from surface water; tracts 5, 6, 7, 10, 13, 14, 15, and 17 will be served from ground water; and tracts 11 and 12 will be served by a combination of drainage and ground waters. The 810 acres to receive a replacement supply are located in Area 4 and would be irrigated with surface water. Presently, they are irrigated with ground water.

The study also evaluates 29 existing tracts of Federal lands which were set aside in conjunction with the development of the North Side Pumping Division for wildlife management by the Idaho Department of Fish and Game. These tracts, which are located throughout the division, include a total of about 1,020 acres.

The climate of the study area is similar to that of the rest of the southern portion of the Snake River Plain. Precipitation is low, averaging only about 10 inches annually. The growing season precipitation is unpredictable and averages only about 4 inches a year. Half the 4-inch total may be received in 1 month, and little or no rain may fall in other months. Summer temperatures are high, and winters are cold. The growing season for hardy agricultural crops averages 190 days, and the frost-free period averages 130 days.

### EXISTING NORTH SIDE PUMPING DIVISION

The North Side Pumping Division was constructed in the 1950's and irrigates 76,800 acres of land. Surface flows of the Snake River and ground water from the Snake Plain aquifer constitute the sources of irrigation water



to the project. Unit A is comprised of 14,600 acres of irrigated land and is served from surface water pumped from the back waters of Milner Dam. Ground-water pumping from the Snake Plain aquifer irrigates 62,200 acres in Unit B. The A&B Irrigation District operates and manages the project.

A portion of the water supply for Unit A is provided by storage in American Falls and Palisades Reservoirs. Water for Unit A is pumped from the Snake River by a pumping plant 8 miles west of Burley. The pumping plant delivers water to a canal system serving the Unit A lands.

A total of 177 deep wells provide the ground-water supply for Unit B. The wells are 12 to 24 inches in diameter, average pumping head is about 200 feet, and the average well discharge is about 6.5 cubic feet per second  $(ft^3/s)$ . Unit B was the first large-scale ground-water pumping project on the Snake Plain. Since it was built, large acreages have been irrigated from ground water elsewhere on the plain.

A distribution system consisting mainly of unlined ditches was originally used to distribute water in both units. In recent years, many irrigators have converted to sprinkler systems.

The project area lacks a well-defined stream drainage pattern because of its youthful stage of geologic development. As a result, the project area has some enclosed drainage basins--relatively shallow depressions with no natural drainage outlets. Project drainage for runoff and return flows in those areas has been accomplished by the construction of drainage or injection wells which are drilled into highly porous zones of the underlying lava rock.

The irrigation of some of the high quality Extension lands is needed to expand the economic base of the area. Based on public input, plus economic, operational, and social needs of the present farm operators, it has been concluded that the most pressing agricultural need is the addition of small or

moderate amounts of land to existing farms rather than the establishment of new farm units.

Many of the existing farms in the North Side Pumping Division are quite small by present-day standards; average ownership in the A&B Irrigation District is about 135 acres. Because of the small farm sizes, some of the farms in the area are marginal or may be losing money under present conditions. On those farms, the addition of a relatively small amount of new land could mean the difference between economic survival and failure. Further, the layouts of some of the existing farm units do not lend themselves to improved farming methods, such as conversion from gravity irrigation to the more efficient types of sprinkler irrigation. In a number of cases, the acquisition of a relatively small acreage of Extension land could permit the conversion to more efficient farm practices.

### CHAPTER 1--WATER RESOURCES

The water supply can be categorized by the source--surface water or ground water. To avoid confusion, the water supply discussion was divided into two sections--surface water and ground water. The surface-water discussion follows, and the lands served from ground water will be discussed in the following chapter entitled "Ground-water Resources."

### Surface-water Supply--North Side Pumping Division

Sources of surface water available to the North Side Pumping Division include Snake River flows, Snake River storage, the "rental pool," and surface drainage water. The United States holds a Snake River natural flow right of 267 ft<sup>3</sup>/s (priority date April 1, 1939) in trust for the A&B Irrigation District. In addition, the district has secured storage water in two of the

seven Reclamation upper Snake River reservoirs. The priority dates and storage space in the two reservoirs are shown in table 1.

Table 1.--Priority Dates and Storage Space

Priority Date	Storage Space
	acre-feet
3/30/21	$46,826\frac{1}{}$
7/28/39	90,800
	137,626
	3/30/21

<sup>1/</sup> Reduced from 47,593 acre-feet in 1978 due to sediment deposit

During the 1973 through 1977 period, American Falls storage was restricted to 66.2 percent of capacity due to dam safety considerations and reconstruction of the facility. Accordingly, the district's storage capacity was limited to 31,495 acre-feet during this 5-year period. Further, the district's American Falls storage right was 47,593 acre-feet prior to 1973; however, subsequent to reconstruction (1978), each shareholder's capacity was revised to reflect the reduction in American Falls capacity due to sediment deposits.

### Surface Water--Snake River

The irrigated area served from the Snake River diversions and its tributaries at and above Milner Dam exceed 1 million acres. Diversions to these lands range from 4 acre-feet per acre or less on pumping projects to 15 acre-feet per acre in areas where subirrigation is practiced. A substantial amount of the diverted water returns to the river, however, and is reused during the same season or is stored in American Falls Reservoir during the winter for irrigation use during the following season.

Historic records indicate the surface-water resources serving the irrigated area above Milner Dam are highly developed. Under present development, the Snake River flow would have been fully controlled and utilized for irrigation except releases required to meet established water rights below Milner Dam during the drought period from 1931 to 1935. The opportunities for expanding the lands irrigated from this source are limited without additional storage in the system.

During periods of below average runoff, the natural flow in the Snake River has been entirely appropriated. During these periods, the district's surface-water right priority date does not allow them to divert natural flow to Unit A. However, in periods of average to above average runoff the district diverts natural flow to Unit A, and records maintained by the district indicate that diversions during these periods can meet all of the Unit A irrigation demands through June and, in very wet periods, the early part of July. The use of natural flow whenever available insures the project's carryover storage will be at a maximum level when entering a dry cycle.

The irrigation of Unit A land with Snake River water as a source of supply is dependent on adequate storage water. To insure a base supply during the dry cycles (regardless of length), the reservoir providing the storage water must have a storage right which guarantees a firm yield (one to one), which American Falls Reservoir provides. Palisades Reservoir, on the other hand, does not have a one-to-one yield; however, it was designed to capture surplus flows during wet cycles and carry storage over for irrigation use during dry cycles.

The Bureau of Reclamation began development of the North Side Pumping Division facilities in the early 1950's, and the project was essentially

complete in the early 1960's. In 1963, the district began and has maintained diversion records for the Unit A Snake River pumping station. For the 1963 through 1983 period, the 21-year average annual surface water diverted to Unit A was 56,400 acre-feet, of which 23,000 acre-feet was natural flow and 33,400 acre-feet was storage. The records indicate Unit A lands have not experienced an irrigation shortage for the 21-year period examined, and there were only 3 years (1966, 1973, and 1977) in which the irrigation diversions exceeded natural flow plus the available storage in American Falls and required drafts from Palisades storage. Two of three years occurred during the 5-year period in which American Falls storage was restricted to 66.2 percent and the district's storage in American Falls was limited to 31,495 acre-feet.

When a spaceholder utilizes Palisades storage, a conveyance loss is charged to the user. The quantity of storage water released from Palisades Reservoir includes this conveyance loss. The district's average annual storage water diverted to Unit A was 33,400 acre-feet; however, because they utilized Palisades storage in 3 of the 21 years examined, the average annual storage water charged (released) was 33,500 acre-feet. In 1978, this practice of charging a conveyance loss to Palisades storage was discontinued.

Storage from Palisades Reservoir (including conveyance losses) required to meet the irrigation requirements of the Unit A lands in 1966, 1973, and 1977 were 8,690, 17,130, and 24,150 acre-feet, respectively. With American Falls Reservoir having a firm yield (one to one), it can be assumed that the Palisades storage requirements in 1973 and 1977 would have been significantly reduced had the storage restrictions on American Falls Reservoir not been in force. Assuming no storage restrictions on American Falls, the estimated

Palisades storage requirements (including conveyance losses) in 1973 and 1977 would have been about 1,800 and 8,820 acre-feet, respectively.

At the beginning of each irrigation season, storage space in the upper Snake River reservoirs is allocated to each spaceholder. During the 1963 through 1983 period, the 21-year average annual storage allocated to the district was 129,400 acre-feet (43,500 acre-feet from American Falls and 85,900 acre-feet from Palisades). As previously discussed, the average annual storage charged to the district over this same period was only 33,550 acre-feet (31,170 acre-feet from American Falls and 2,380 acre-feet from Palisades. Had American Falls storage not been restricted to 31,495 acre-feet (about a 15,330-acre-foot reduction) from 1973 through 1977, the average annual American Falls allocation would have been about 47,200 acre-feet. Further, the average annual storage required from Palisades Reservoir would have been reduced from 2,380 acre-feet to about 900 acre-feet (about 1 percent of the average annual Palisades storage allocated).

The district has maximized their natural flow diversions, and when storage has been used to meet the irrigation requirements of the Unit A lands, the district has utilized American Falls storage first and Palisades storage second. This practice has maximized annual carryover storage because the district receives their full American Falls storage allocation about 98 percent (estimated) of the time, and when storage has been required, the American Falls storage is capable of meeting the majority of the annual Unit A diversion requirements. In the 3 years Unit A storage requirements exceeded the available American Falls storage, between 84 and 96 percent of the annual storage requirements would have been provided by American Falls assuming no storage restrictions during the 1973 through 1977 period.

Under the proposed project, 3,950 acres would be provided a full water supply in Unit A, and a portion of the annual carryover storage would be utilized to meet the irrigation requirements of these new lands. Examining the district records and assuming the new lands would utilize available American Falls storage first, the estimated 21-year average annual available American Falls storage to the proposed new lands was 15,100 acre-feet, of which an average of approximately 10,000 acre-feet would be utilized. Storage requirements for the new lands not satisfied by American Falls storage would be met from Palisades storage. Based on the historic records, the estimated 21-year average annual Palisades storage requirements for the new Unit A lands would be less than 2,500 acre-feet. The estimated 21-year average annual Palisades storage requirements for the existing lands plus the proposed new lands would represent about 5 percent of the 21-year average annual Palisades storage allocation. The district's records indicate it is possible to serve the proposed new Unit A lands with available storage from American Falls and Palisades Reservoirs and still maintain adequate carryover storage to assure a dependable water supply during dry cycles to all (existing and new) Unit A lands.

There is one additional potential source of Snake River water which has not been addressed—the rental pool. Rental pool water is made available by spaceholders when they determine storage water allocated to them is in excess of their needs. This water is offered to the rental pool by the spaceholder, at which time the "rental pool committee" determines if there is a need for the water offered and further who might use it. Individuals or canal companies who forecast their need for water greater than their ownership rights may apply for additional water from the rental pool committee on an annual or long-term (through lease agreements) basis. Based upon the decision

of the committee (which is comprised of representatives from the "Committee of Nine," Reclamation's Minidoka Project office, and the watermaster), the water is made available to the interested applicant for a fee. The fee in 1984 was \$2.50 per acre-foot of water.

Assuming the new irrigation demands require the A&B Irrigation District to utilize the rental pool, the quantity that might be used would not adversely affect the other users of the pool. There are no priorities for use of the rental pool, only the annual determination by the committee. The cost of water from the rental pool would be covered with the annual operation and maintenance charges made to the project. Based on the historical records, it appears highly unlikely this potential source of water will be required or utilized under the Recommended Plan.

### Surface Water--Drainage

The proposed plan would develop a partial solution to the injection well problem by irrigating Extension lands from existing North Side Pumping Division drains wherever possible. There is a potential of using drainage water to satisfy part of the irrigation water requirements of several blocks of new land located near the terminus of major drainage ditches in the A&B Irrigation District Beginning in 1980, Reclamation, in conjunction with the A&B Irrigation District, began monitoring surface drainage flow during the irrigation season. In each year subsequent to 1980, several additional new measuring sites were established on injection wells and drainage ditches leaving the project.

The available records at each site vary from 1 to 4 years and indicate that the majority of the drains have periods during the irrigation season of minimal (zero) flow. There are substantial fluctuations in the daily discharge, and, to maximize the use of the drain flows, small regulating

facilities capable of storing the daily irrigation water requirements of the new lands would be required on the majority of the drains. On several of the drains, the size of the regulating facility required to maximize drainwater use would inundate an area of existing farmland which is approximately the same size as the area of new land proposed for development. Further, the annual yield on the majority of the drains will not meet the annual irrigation water requirements of the proposed new lands.

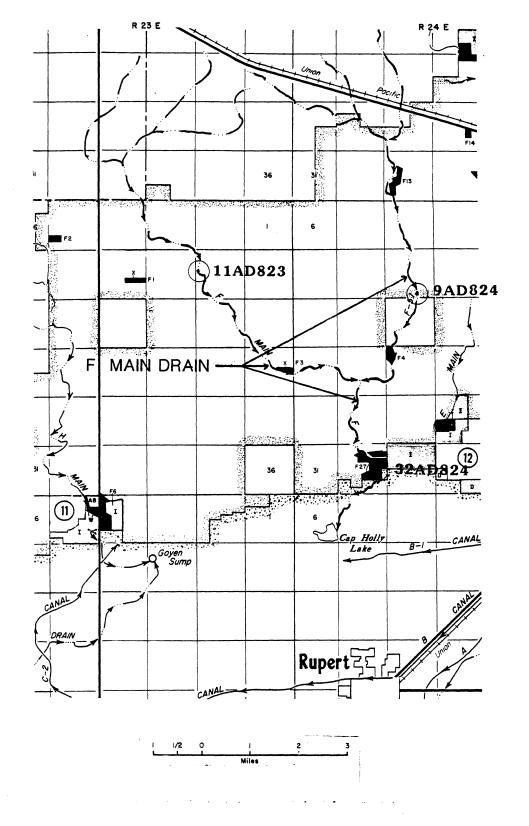
There are two areas where use of drainage water was found feasible as a potential source of water--tracts 11 and 12 (see Proposed Land Uses map). Drainage water will be diverted to Area 11 from the H Main Drain, and Area 12 will utilize drainage water from the E and F Main Drains. Records for the H Main Drain indicate this drain is capable of providing part of the irrigation water requirements of Area 11 (120 acres), and the records for the E and F Main Drains indicate there generally is sufficient drainwater to provide a dependable water supply to the land located in Area 12 (460 acres); however, some supplemental pumping from the existing project wells may be required in the early spring and late fall.

In 1982, a data collection site was established on the H Main Drain. There are two injection wells (4BD923 and 5AD823) on this drain, and the collection site was established between the two wells. In 1983, the collection site was relocated upstream of both injection wells so the total available drainwater could be estimated. The limited data available at the two sites indicate the H Main Drain can provide part of the irrigation water requirements to Area 11; however, a new ground-water well will be required to augment the drain flows for much of the irrigation season. Based on this limited data, the estimated drainage water which can be utilized by Area 11 is about 100 acre-feet per year (average).

Currently, about 70 acres in Area 12 are leased from the Government and are irrigated from drainwater in the E Main Drain. The estimated drainage water diverted from the E Main Drain is about 200 acre-feet per year (average), and it was assumed this practice would continue when the project is developed. The remaining 390 acres in Area 12 would be served from the F Main Drain.

Data collection on the F Main Drain began in 1981. There are three injection wells on this drain (11AD823, 9AD824, and 32AD824), and in the 1981 and 1982 seasons, the data collection site was located below all three wells. The total amount of water in the F Main Drain could not be quantified during the 1981 and 1982 seasons because the volume of water entering the three wells was not recorded.

In 1983, the collection site was relocated to just upstream of drain well 32AD823, and a new collection site was established to monitor the flows entering injection well 9AD824. The map of F Main Drain on the following page shows the location of the three injection wells on F Main Drain, noting the position of the recorder sites (above 32AD824 and at 9AD824) still does not account for the water entering injection well 11AD823. The 1983 data collected at the site just above drain well 32AD824 indicates there is sufficient drainwater in F Main Drain to provide an adequate water supply for 390 acres in Area 12 from mid-June through the end of August and would only require a very small regulating facility. In the period prior to mid-June and subsequent to August, the water entering the injection wells 11AD823 and 9AD824 would be diverted to Area 12 if water shortages occur. In the event the new lands would experience a shortage in the early spring to late fall, the existing project wells could be utilized to insure an adequate water supply for this period. The limited data available indicate the estimated



# F MAIN DRAIN

- INJECTION WELL
- 1 -EXTION STUDY AREA NUMBER

# **EXISTING IRRIGATION**

A&B IRRIGATION DISTRICT (NORTH SIDE PUMPING DIV.)

- UNIT B, GROUND WATER

MINIDOKA IRRIGATION DISTRICT (GRAVITY DIVISION)

- SNAKE RIVER SERVICE

drainage water which would be diverted from the F Main Drain to Area 12 would be about 900 acre-feet per year (average).

### CHAPTER 2--GROUND-WATER RESOURCES

### Introduction

A primary purpose of this chapter is presenting geohydrological data to aid planners in irrigation well-site selection and construction design for the North Side Pumping Division Extension studies.

Ground-water pumping for irrigation began in southern Minidoka County in 1947. The Bureau of Reclamation drilled three irrigation wells in 1948 and leased them to private landowners for irrigation. These wells proved the feasibility of ground-water pumping for irrigation and were the forerunners of numerous irrigation wells in the area.

Unit B contains about 62,200 acres, irrigated with ground water pumped from 177 wells located throughout the project area. Records maintained by the district indicate the 20-year average (1963 through 1982) annual ground water pumped to Unit B lands was 208,240 acre-feet.

A water level decline, beginning in the 50's and thought at the time to be caused by ground-water pumping, resulted in about 90 wells being deepened. Deepening began in about 1958 or 1959, and continued until about 1964. A few wells were deepened twice. Several wells were deepened immediately after completion because of inadequate yield. One well was deepened in 1981.

The ground-water requirements for the proposed new Unit B lands will be provided from 18 ground-water wells (14 new and 4 existing) located near or within the individual tracts of land. During original development, five wells were drilled and not used. One well, 33A824, has been sold to the city of Rupert. The remaining four wells would be used for a water supply. Some or

all of these wells may require some renovation to bring them up to current Reclamation well construction standards.

### Conclusions

Net ground-water pumping for the proposed North Side Pumping Division Extension project is expected to increase total pumpage from the area by approximately 13,520 acre-feet per year--about a 2.5-percent increase over present use in the Snake Plain aquifer in the Rupert to Thousand Springs area and about a 1-percent increase for the entire Snake Plain aquifer. The plan would provide for 15,620 acre-feet of additional pumpage but would be offset by 2,100 acre-feet for the replacement area (810 acres).

The major influence upon ground-water level declines and recoveries is climate; therefore, the increased pumpage is not expected to have a measurable effect upon present water levels. Locally, a water level drop of 1 to a few feet may occur but, because of the enormity of the Snake Plain aquifer and the ground-water supply, the effect will be difficult to identify at any distance beyond the project area. The effect of the additional pumpage, a possible 0.25-percent decrease in aquifer discharge at Thousand Springs, will not be identifiable at the springs.

### Geology

The project area topography consists of gently rolling hills typical of a basalt flow landscape. This is in contrast to the relatively flat river formed landscape to the south. An abrupt rise in surface elevation marks the southerly extent of lava flows. This topography change is the dividing line between the Minidoka Irrigation District to the south and the North Side Pumping Division Irrigation District on the north.

A blanket of wind-deposited silt, sandy clay, and very fine sand from a few inches to a few tens of feet thick covers most of the underlying basalt.

There are, however, a few isolated basalt outcrops dotting the area. In other areas, basalt debris at the surface testifies of more basalt at shallow depths.

The basalt is typical of Snake River basalt being made up of everything from ash, cinders, and highly fractured rock to thick, dense, and massive flows. The sediment is mostly clay and silt with some coarse sand and little gravel.

The subsurface geology of the Minidoka Irrigation District is comprised predominantly of sediment with the amount of sediment decreasing east, west, and north. Nearly all the area beneath the North Side Pumping Division Unit B is made up of basalt with few to minor amounts of sediment. The basalt is mostly highly to moderately broken with many innerflow zones of rubble and/or cinders. The subsurface beneath tract 4 is composed of basalt innerbedded with substantial amounts of mostly fine-grained sediment. The subsurface geology of the area occupied by North Side Pumping Division Unit A is also primarily basalt with minor amounts of sediment locally.

### Geohydrology

# Snake Plain Aquifer

The ground-water source for the North Side Pumping Division is the extensive Snake Plain aquifer; hence recharge and/or withdrawal changes within the Snake Plain aquifer affects the North Side Pumping Division aquifer, and recharge and/or withdrawal changes within the North Side Pumping Division aquifer affect the Snake Plain aquifer, primarily aquifer users down gradient from the North Side Pumping Division. Therefore, a look at the Snake Plain aquifer is essential to understanding the ground-water regime of the North

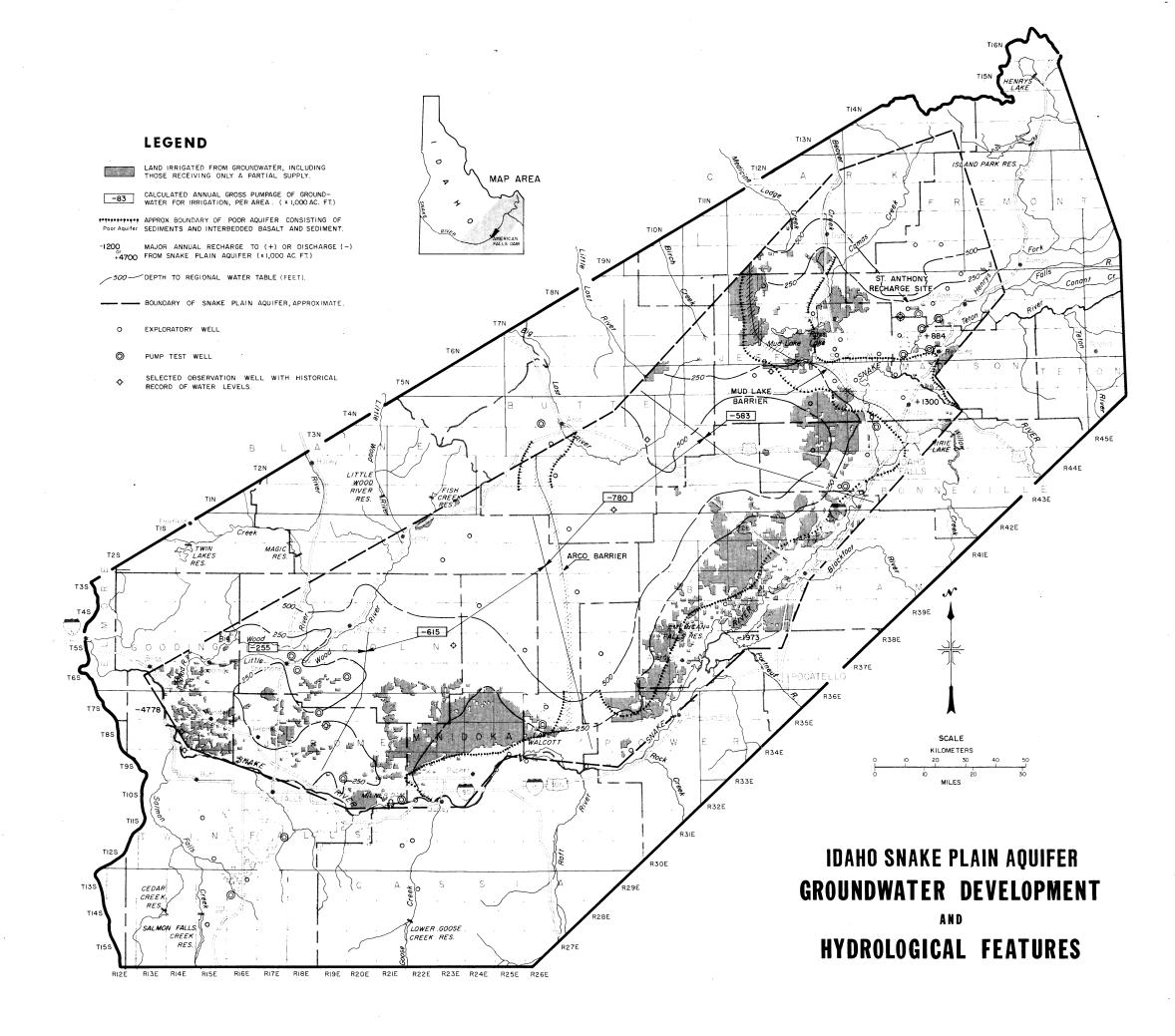
Side Pumping Division. The following is a discussion of the Snake Plain aquifer from "Investigations Ground-Water Supply for Salmon Falls."

The Idaho Snake Plain aquifer (see map on following page) is one of the largest and most prolific in the Nation. The aquifer extends from St. Anthony to Bliss, a distance of 180 miles, and averages 60 miles wide. Area extent is about 10,800 square miles or 6,900,000 acres.

The trough which contains the Snake Plain aquifer is a broad downwarp or downdropped fault block extending over 190 miles from Bliss to Ashton. Rocks which form the bordering mountains and the floor of the trough are largely consolidated materials of low permeability. The trough is filled with a thick sequence of lava flows including basalts of the Snake River Group. Major sites of outpouring of lava appear to have been concentrated in three areas of the plain: An area north of Rupert; on the plain between Blackfoot and Arco; and an area south of Dubois. There is evidence that volcanic activity in the three areas was progressively younger from west to east. The concentrations of lava accumulations tend to be encircled by sediments.

As the trough filled, the ancestral counterparts of present-day streams were repeatedly diverted, confined, and ponded. The successive layers of basalt in areas of volcanic activity continuously maintained higher elevations and stream courses were restricted to lower topography around the margins of the accumulating lava flows. Sediments from ancient streams and lakes accumulated to great depth in approximately the same location as present streams.

The Idaho Snake Plain Aquifer map shows a boundary between areas of predominantly basalt and areas of alluvium, in part interfingered with basalt. In most areas of surface or near-surface sediments such as at Rupert, the upper end of Walcott Reservoir, American Falls Reservoir, and at Mud Lake, the



deepest drilling of about 1,000 feet is still in alluvium. Several deep wells in the Mud Lake area penetrate varied clays at about 800-feet below surface showing that a lake existed in that area in glacial time. Most of the sediments are fine grained and poorly permeable although there are occurrences of sand and gravel.

The occurrence of poorly permeable lake sediments such as at Mud Lake, extending across the aquifer, in effect form a "dam" which segments the aquifer. The dam formed by the Mud Lake sediments is referred to as the Mud Lake barrier. The water table above the barrier is at about elevation 4700 and drops to 4550 immediately below. A similar zone of steepened gradient extends across the plain between Arco and Lake Walcott (Crosthwaite, 1973). This is interpreted as a barrier of sediments similar to the Mud Lake barrier and is noted as the Arco barrier on the Snake Plain Aquifer map. The water table above the barrier is at elevation 4400 and descends to elevation 4100 below the barrier in a distance of 3 miles.

Although the Snake Plain aquifer appears to be segmented into three separate compartments or reservoirs, there is a degree of hydraulic continuity. The effect of the aquifer dams is not fully understood at this time but may be significant in regard to location and effects of recharge or withdrawals from the aquifer. Water table changes occurring in one segment appear to be dampened in passing through a downgradient barrier.

Thickness of the Snake Plain aquifer is unknown except in marginal areas of the plain. The deepest drilling on the plain is about 10,000 feet. Present interpretation of a geophysical profile developed by the Geological Survey between Blackfoot and Arco indicates a maximum thickness of 7,000 feet. There are indications that permeability may decrease with depth so that the maximum effective aquifer thickness may be considerably less than 7,000 feet.

Transmissivity  $\frac{1}{2}$  of the aquifer ranges upward to 36 x  $10^6$  gpd/ft as measured in controlled pump tests. Values approaching  $100 \times 10^6$  gpd/ft are suspected under some interior portions of the plain to accommodate the known magnitude of flow. In comparison, transmissivity values for the Columbia River basalts are in the range of 12,000 to 30,000 gpd/ft in eastern Washington, and 67,000 to 130,000 gpd/ft in the Umatilla, Oregon, area. At The Dalles, Oregon, transmissivity of the Yakima basalt ranges upward to about 300,000 gpd/ft.

A true value for storativity  $\frac{2}{}$  has not been accurately derived for basalt aquifers from relatively short-period pump tests. Most investigators have used a range of storage values for the Snake Plain aquifer of between 10 and 20 percent. Even using the smaller of the two, a change in water table of 1 foot across the plain represents a change of water in storage of 690,000 acre-feet.

The aquifer is fed by seepage from streams which enter or cross the plain, underflow from tributary valleys, irrigation return flows, and from precipitation on the plain and bordering foothills. Prior to the advent of large scale irrigation about 3 million acre-feet per year moved through the basalts in this way--presently recharge from surface-water irrigation amounts to about an additional 4.8 million acre-feet per year. Discharge from the aquifer occurs as spring flows concentrated near the upper end of the American

<sup>1/</sup> Transmissivity indicates ability of aquifer to transmit water--gallons per day which would move through a 1-foot wide slice of the aquifer under unit gradient

<sup>2/</sup> Storage or storativity is an indication of effective porosity of an aquifer--the volume of water an aquifer releases or acquires per unit surface area of the aquifer per unit change in head. At 10 percent storage, a water table rise or decline of 10-feet under 1 acre would equal a gain or loss of 1 acre-foot of water; and at 20 percent storage a gain or loss of 2 acre-feet.

Falls Reservoir and at Thousand Springs, and as ground-water pumpage for domestic, municipal, and irrigation supplies.

The Inflow, Outflow, and Storage map on the following page illustrates the Snake Plain aquifer water budget.

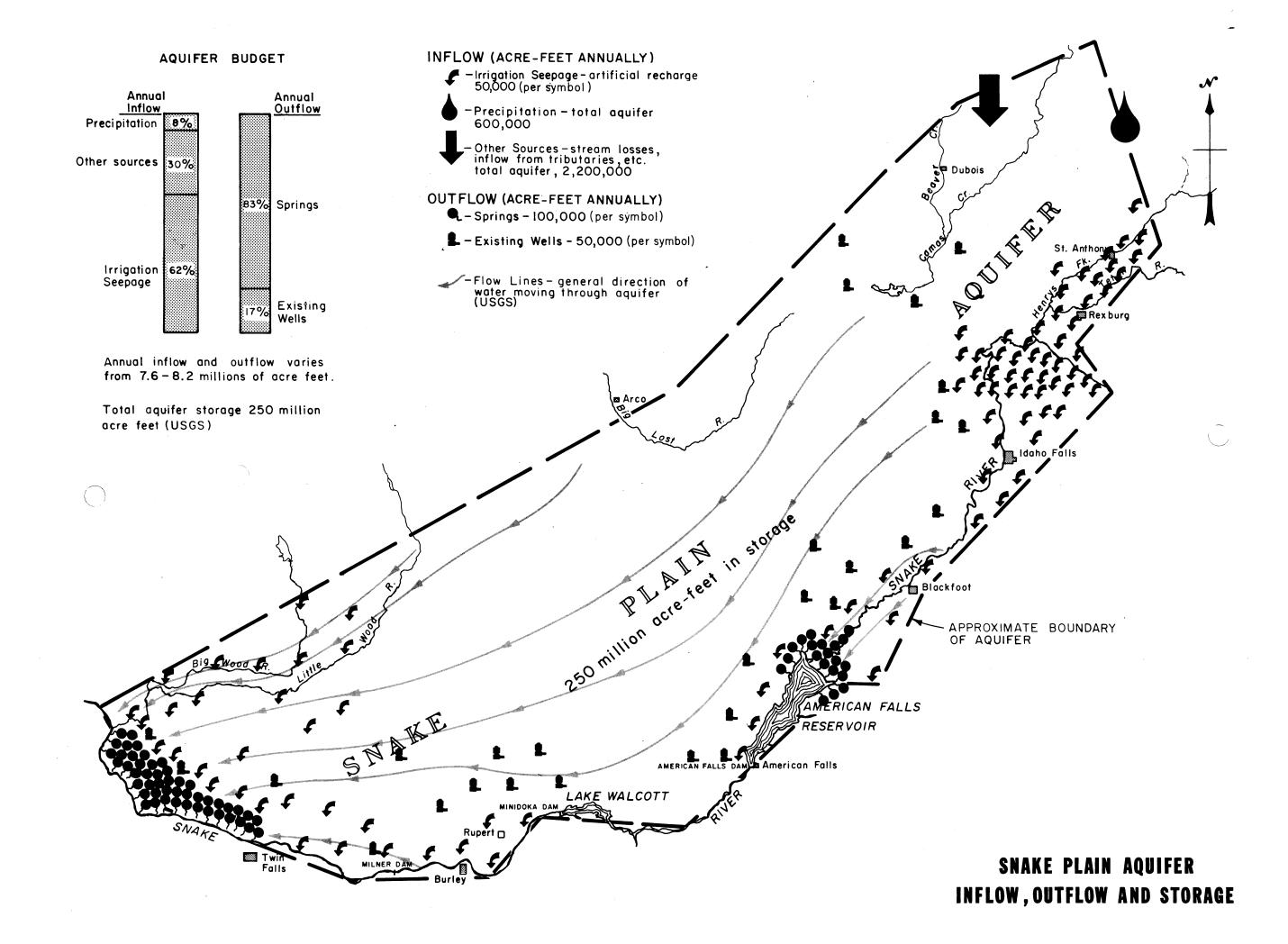
North Side Pumping Division Aquifer. -- The aquifer, as previously discussed, is made up of sediment and basalt. A few locations, particularly part of that area occupied by Minidoka Irrigation District, have wells that penetrate no basalt; other locations have wells with little or no sediment.

The sediment is mostly fine grained with some sand and little gravel.

Water yield from the sediment is generally small, up to a few tens of gallons per minute.

The basalt is made up of a series of thin flow sheets, from a few feet to several tens of feet thick. Where the flow sheets are deposited one upon another to form a relatively thick sequence, and where the basalt is highly fractured and/or contains numerous rubble or cinder zones, the water yield is large, up to several thousand gallons per minute. Where the flow sheets are made up of dense, and massive basalt and/or is covered, penetrated, or innerbedded with fine sediment, the water yield is small to moderate. One such area is in the southwest part of Unit B located mostly in T9S/R22E where several low yielding wells are found.

Here the aquifer is comprised of basalt innerbedded with substantial amounts of fine sediment. Some of the basalt in the upper part of the aquifer also contains fine sediment that reduces the permeability. The deeper basalt is relatively free of sediment, but must be thick, massive, and dense with a low permeability because water yield remains low despite more than 100 feet of exposed basalt aquifer in some wells.



The specific capacity of a well is a measure of the yield potential of that well and of the aquifer at that well site. Specific capacity is calculated by dividing the yield in gallons per minute by the water level drawdown in feet. The resultant value is, therefore, reported in gallons per minute per foot of drawdown, or, the aquifer should yield the calculated number of gallons of water each minute for each foot the water level is drawn down. The larger the specific capacity number the greater the yield potential for the well. The Specific Capacity map (map 1 at end of appendix) is a contoured illustration of the specific capacity values for the North Side Pumping Division area. The sediment portion of the aguifer penetrated by the wells was ignored because water yield from the sediment is small, probably not over a few gallons per minute. These specific capacities were derived from reported district well pump test data. The predominantly basalt aquifer has the highest specific capacity, sometimes as high as several thousand gallons per minute per foot of aquifer drawdown; whereas, the predominantly sediment aquifer has the lowest specific capacity, often as low as several gallons per minute per foot of drawdown.

The district well pump tests were run to determine whether the well would deliver the required yield for that site and were not run to determine hydrologic properties of the aquifer. Because of this, inspection of the Specific Capacity map will show a great diversity of specific capacity values throughout the irrigation district. Sometimes adjacent wells have greatly different specific capacity values, one high and the other low. The high value probably more nearly reflects the yield potential for the aquifer; therefore, the large value was plotted on the map.

A Depth to Water map (map 2 at end of appendix) was prepared from district measured water level data (spring 1981) and from surveyed well

elevations. The water table depth is about 80 feet below land surface on the southeast edge of the irrigation district and increases to about 400 feet on the west edge. The increasing depth results mostly from falling water table elevations east to west.

A Water Table Elevation map (map 3 at end of appendix) was drawn from spring 1981 district measured water levels and illustrates ground-water flow direction and ground-water recharge areas. Generally, the ground-water flow sweeps across the North Side Pumping Division area from northeast to west-southwest. Ground-water recharge from Raft River and Oakley Fan and especially from Burley Irrigation District and Minidoka Irrigation District areas flows in a west-northwest direction and is intercepted by the Snake Plain aquifer near the southern edge of the North Side Pumping Division area.

The water table elevation is about 4090 feet on the southeast edge of the North Side Pumping Division area and falls to about 3840 feet on the west edge of the district. This a drop of about 250 feet as the ground water crosses the district. The gradient is quite flat, less than 3 feet per mile, from the east edge of the district to a line running northerly between Burley and Rupert, then swinging in a westerly arc to north of Paul and finally trending northwesterly across the district (see Water Table Elevation map). West of the above line the gradient increases to about 15 feet per mile except for a circular flat area, with a gradient of about 1.5 feet per mile, covering about 25 or 30 square miles near Burley and Paul. The very flat area near Burley and Paul is probably the location of an almost all sediment aquifer.

### Recharge

Because the aquifer beneath the North Side Pumping Division is part of the Snake Plain aquifer, most of its recharge is from that source. The three main recharge sources are precipitation, and surface-water losses from streams and irrigation. Precipitation amounts to about 10 percent of the recharge, loss from streams and other surface-water sources amounts to about 30 percent, and irrigation contributes about 60 percent. Secondary recharge sources, such as drain wells, originate locally.

Snake River flow, loss or gain, between the Neeley Gage below American Falls Dam and the Minidoka Gage below Minidoka Dam has been computed for the period 1908 to 1980. Figure 1 is a plot of the loss-gain data. Losses were initially large, with a high in 1910 of about 381,000 acre-feet, and thereafter, decreased to a low in 1933 of about 1,000 acre-feet. The 1908 to 1933 period had 7 years of flow gain with a high in 1929 of about 108,000 acre-feet and a low in 1928 of about 14,000 acre-feet. Since 1935 there have been only 2 years of flow loss, 1964 with about 4,000 acre-feet and 1967 with about 24,000 acre-feet. During the 1935 to 1980 period there were large flow gains from 1972 to 1975 and in 1980 with a high of about 416,000 acre-feet and a low of about 324,000 acre-feet.

Hydrologists have been unable to positively identify the cause of the change from flow loss to flow gain. The effect of the flow change, however, is not apparent on the water level record. Much of the change occurred before North Side Pumping Division construction and during a period of increasing ground-water recharge from growing surface irrigation diversion throughout the Snake Plain aquifer area that peaked during the 1950's. See figure 2-A for a plot of surface-water diversions on the Snake Plain from 1928 to 1983.

Burley Irrigation District and Minidoka Irrigation District utilize Snake River water, diverted from Lake Walcott, as an irrigation supply. Both districts are ground-water recharge source areas for the Snake Plain aquifer beneath the North Side Pumping Division. The Burley Irrigation District is located south of the Snake River and the Minidoka Irrigation District is

# SNAKE RIVER Computed Flow Gain or Loss Neeley Gage to Minidoka Gage X 1000 Acre Feet

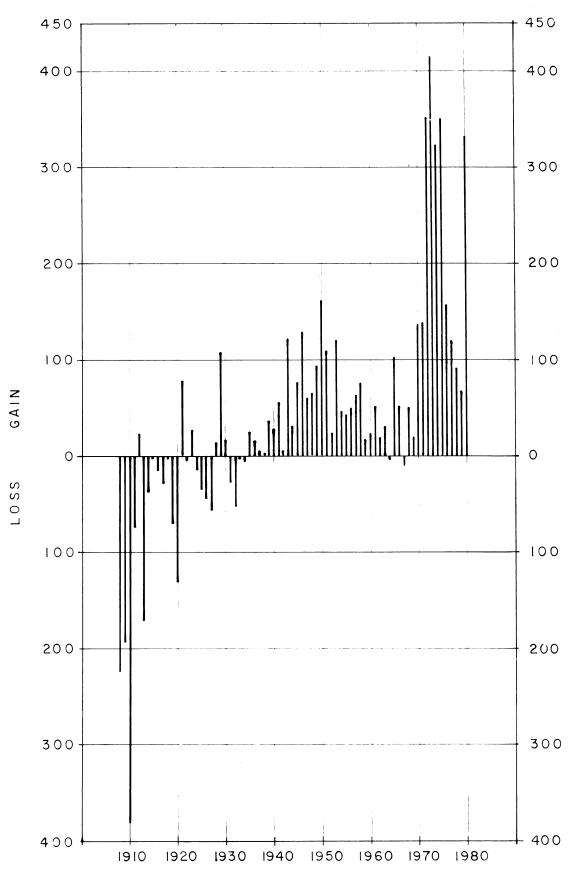


Fig. 1

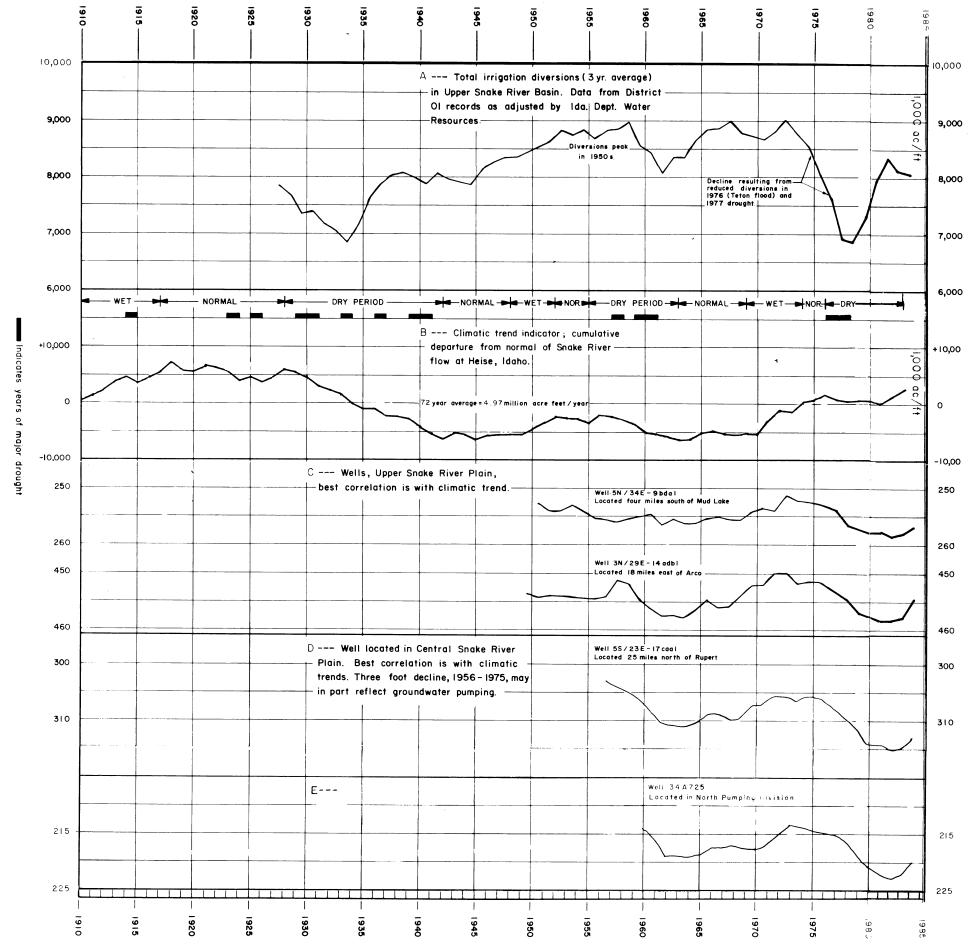


Fig. 2

located north of the Burley Irrigation District between the Snake River and the North Side Pumping Division Irrigation District.

Figure 3-A and B are graphs illustrating Burley Irrigation District and Minidoka Irrigation District's irrigation diversions for a 22-year period from 1960 to 1982. The Burley Irrigation District irrigation diversions, figure 3-A, have remained fairly constant at about 240,000 acre-feet per year for the 22-year period. Whereas, the Minidoka Irrigation District irrigation diversion, figure 3-B, rose from 1960 to a high in 1965 and 1969 of about 580,000 acre-feet each year and then decreased to a low in 1977 of about 414,000 acre-feet and in 1982 of about 421,000 acre-feet.

The Minidoka Irrigation District irrigation diversions decrease as a result of a district water saving program practiced by the irrigators. This diversion decrease represents a sizable reduction in ground-water recharge for that portion of the Snake Plain aquifer feeding the North Side Pumping Division irrigation wells.

Due to the lack of data, an unknown amount of recharge to the aquifer comes from drain wells located throughout the project area. The amount of recharge is not large, probably not over a few thousand acre-feet per year.

Discharge

Discharge from the aquifer occurs as spring flow or from ground-water pumping. Ground-water pumping is the major aquifer discharge in the North Side Pumping Division area, with about 210,000 acre-feet pumped each year within the project. Figure 3-C illustrates district ground-water pumping from 1963 to 1982. Additional ground-water pumping of an estimated 400,000 acrefeet occurs in the area adjacent to and near the project, making this area one of the most intensely pumped on the Snake Plain.

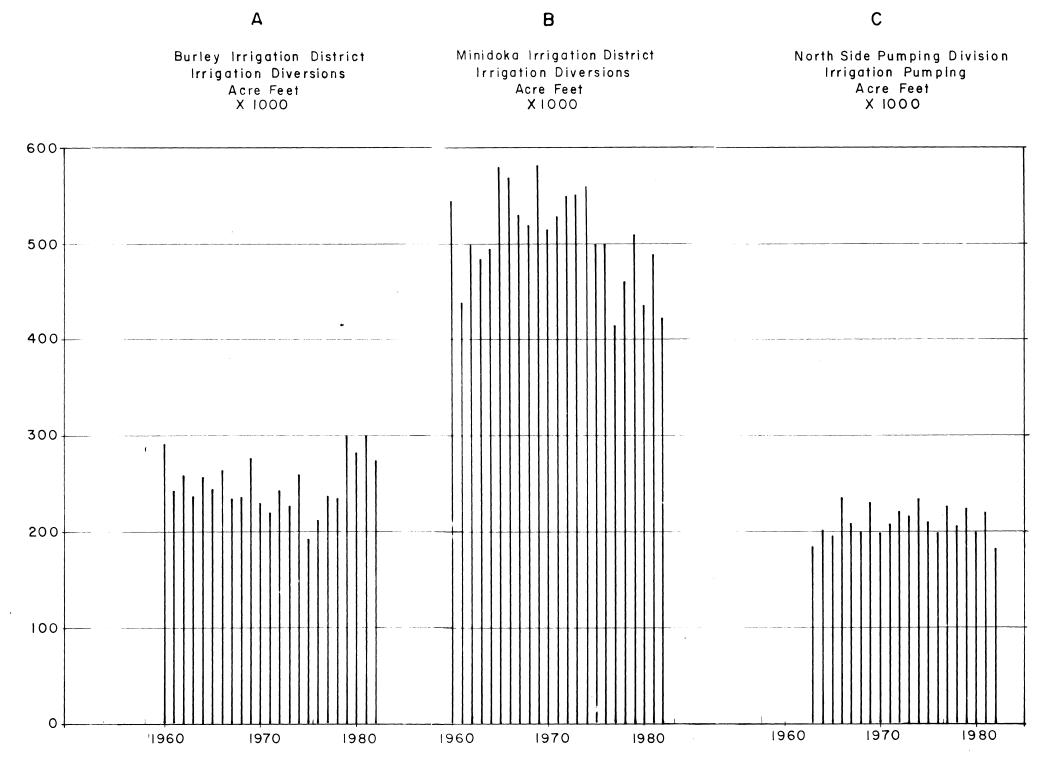
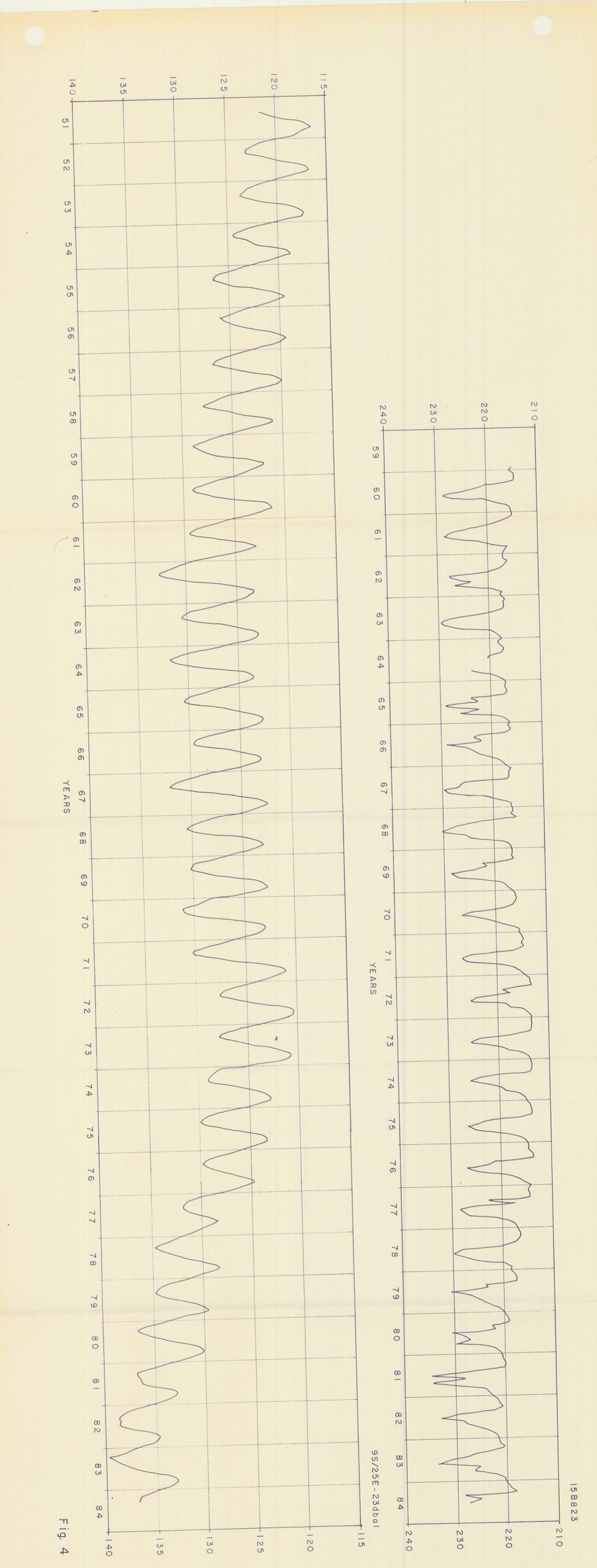


Fig. 3

Water Level Fluctuations.--Water level fluctuations are the aquifer's natural response to changes in storage. These fluctuations include: short-term (seasonal cycle) water table movement resulting from day-to-day changes in aquifer recharge or withdrawal over a period of a year; and long-term water table movement superimposed upon the short-term movement resulting from major changes in recharge or withdrawal extending over a period of several or more years. See figure 4 for illustration of long- and short-term water level fluctuation.

The short-term response is caused by changes in recharge from precipitation, river and reservoir losses, irrigation, and from withdrawals such as pumping. Short-term (yearly) fluctuations in the North Side Pumping Division range from about 4 feet in the porous basalt aquifer in the northern part of the project area to about 20 feet in the sediment, sediment-basalt aquifer in the southern and southwestern part of the project.

The long-term response is caused by such things as major precipitation changes over a period of several years, or by major surface irrigation and ground-water pumping changes. The most significant long-term change appears to be from precipitation. Comparison of the long-term hydrograph in the North Side Pumping Division area (figure 2-E, well 34A 725) with the Snake River flow hydrograph (figure 2-B) from the Heise gage show very close correlation. The Heise gauge was selected because it is upriver from major irrigation diversions and thus reflects changes in precipitation instead of changes in irrigation diversions and return flows. Well hydrographs located downstream from the Heise gage reflect a combination of influences from irrigation and precipitation. Therefore, similarities in the Heise River gage and downstream well hydrographs are caused by precipitation.



The Water Level Change maps (maps 4 through 7 at end of appendix) are contoured plots of water level change within the North Side Pumping Division area. The long-term water level peaked in the early to mid-1950's and then began a water level decline period lasting until the early to mid-1960's.

Because of the water-level decline (1950's to 1960's), ground-water geologist Jack Frink was hired to study the ground-water problem. During this decline period, there was about an average 12-foot aquifer water level drop. The 1953-58 to 1963 Water Level Change map (map 4 at end of appendix) illustrates declines ranging from 1 to 36 feet. In an effort to determine the cause of the decline, water-level hydrographs and other data from throughout the Snake Plain aquifer area were studied. The hydrographs included those within the North Side Pumping Division and other irrigated areas as well as those beyond direct influence of pumping or surface water irrigation. Mr. Frink estimated that 6 to 7 feet of the decline was probably caused by irrigation pumping and the remainder was caused by a drier climatic period.

A wetter climatic period from the mid-1960's to the early to mid-1970's, followed the 1950's to 1960's declining period and caused the water levels to recover an estimated 5 feet of the 12-foot decline. The 1963 to 1973 Water Level Change map (map 5 at end of appendix) shows recoveries ranging from 1 to about 15 feet. Another declining period began in the 1970's and continued until about 1982 (1973 to 1982 Water Level Changes map in pocket). There has been an estimated net 10- to 15-foot decline from the 1950's to 1982 (map 7 at end of appendix). This decline resulted from a combination of causes including: (1) a drier climatic trend in the 1950's to 1960's and 1970's to 1980's; (2) additional ground-water pumping throughout the Snake Plain aquifer area, with over 400,000 acre-feet in the area adjacent to the North Side Pumping Division; (3) reduced wintertime diversions beginning in the early

1970's; (4) reduced irrigation diversions in the Minidoka Irrigation District resulting from water savings practiced by the irrigators; (5) increased pumping in some of the North Side Pumping Division aquifer recharge areas such as Northwest Raft River and Northern Oakley Fan.

Water level hydrographs (figure 2C, D, and E) show the 1970's to 1982 water level decline has stopped and is showing a slight recovery for 1983 and a stronger recovery for 1984. If this recovery continues a portion of the past decline will be regained.

Additional ground-water pumping has slowed on the Snake Plain because of water rights controversy and because most potential irrigable land is in production. The portion of the decline caused by pumping has slowed and may have stabilized.

Comparison of four water level hydrographs (figure 2C, D, and E) spaced along the Snake Plain show that about 6 to 9 feet of the decline between the 1970's and 1982 was probably caused by climate and other causes such as reduced diversion and any additional decline may be caused by pumping. The total decline may be slightly over 9 feet in the project area. In the southwest part of the project area, hydrographs (figure 5) show the decline slowing and may have stabilized or is showing a very slight recovery. The reason the water levels in this area are behaving differently than elsewhere on the project may be due to ground-water pumping in Northern Oakley Fan.

The total net decline in the project area from all causes is about 5 feet from the 1963 low to the 1982 low.

The 1970's to 1982 water level decline period resulted in dropping water levels that caused pump and water yield problems throughout the North Side Pumping Division area. This caused renewed concern about ground-water mining among area pumpers, and stimulated demands for ground-water pumping

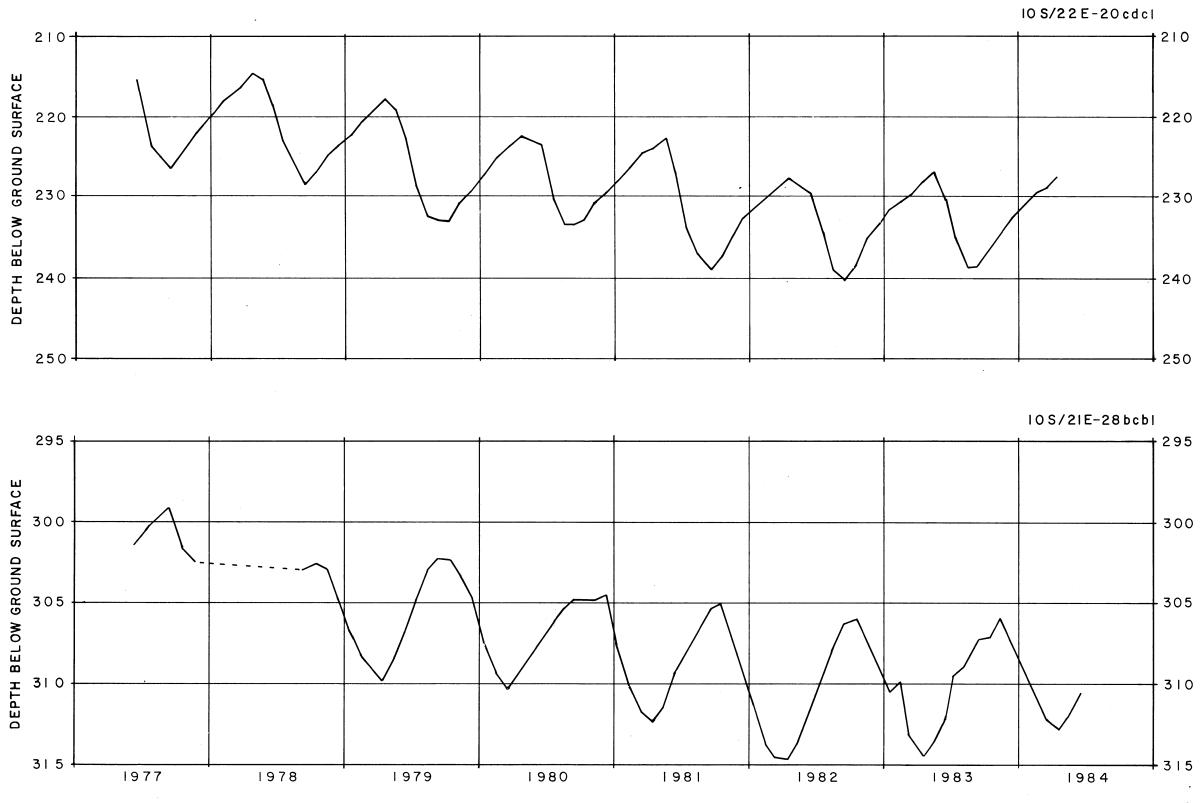
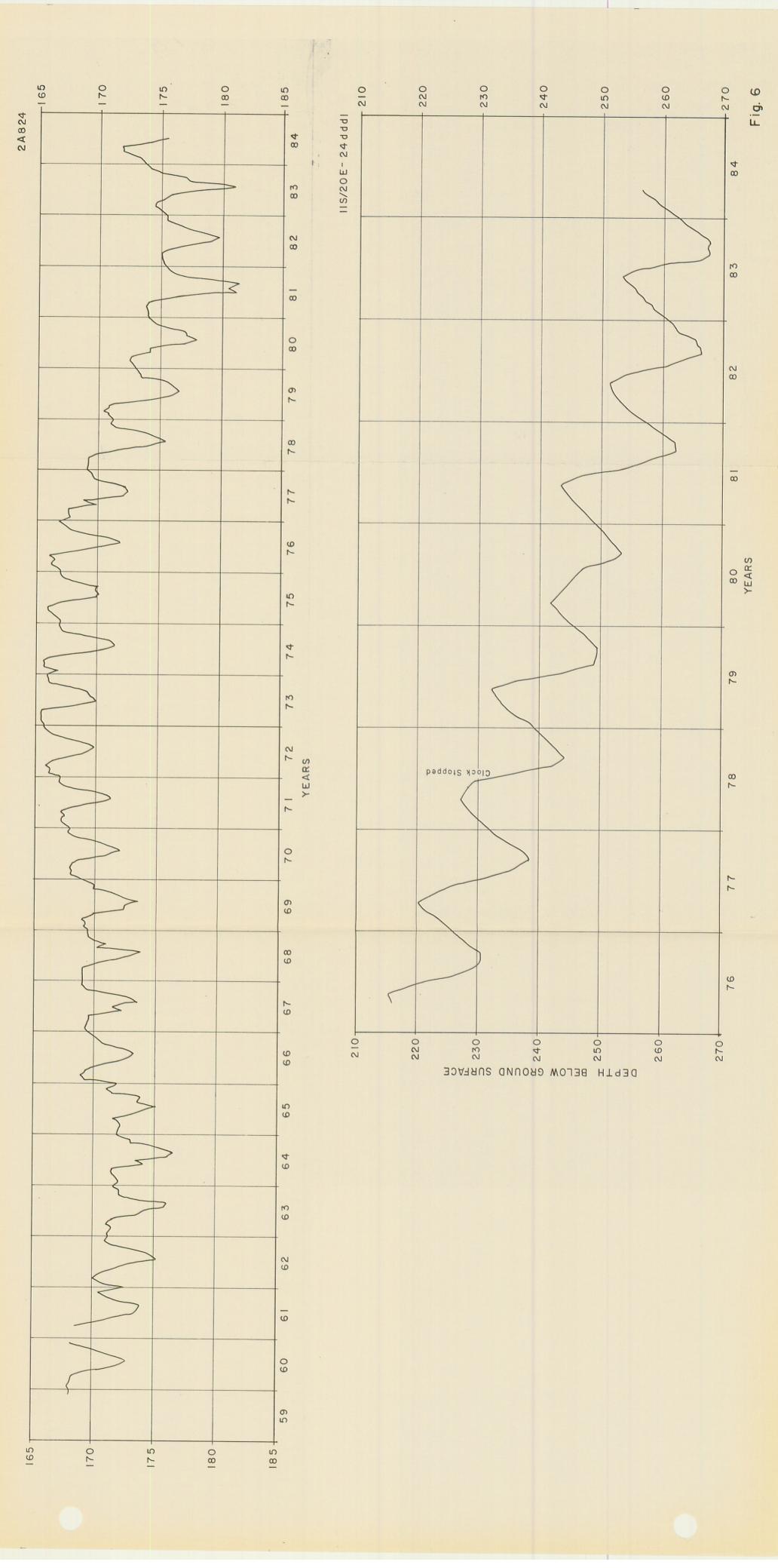


Fig. 5

restrictions. Reclamation was again requested to study the problem. Review of the previous decline data found the conclusions sound. The current decline problem like the previous one was mostly related to a drier climate trend, but also aggravated by changes in irrigation practices, such as reduced irrigation diversions, throughout the Snake Plain aguifer area. It was concluded that the water level decline was not permanent and, because of a wetter climate trend beginning in about 1983, appears to have begun a recovery. Fear of ground-water mining was also unfounded as shown by comparing water level behavior in the North Side Pumping Division area with that from a local area where ground-water mining is occurring. See figure 6 for comparison of a North Side Pumping Division water level hydrograph (well 2A824) and a ground-water mining hydrograph (well 11S/20E-24ddd1). Both hydrographs show yearly fluctuations dropping in spring or early fall and rising in fall or early spring, reflecting influences from major short-term recharge and discharge. The main difference between the hydrographs is shown on the long-term trend. The North Side hydrograph shows decline and recovery, whereas the ground-water mining hydrograph shows a continuing decline of about 40 feet in 9 years with no evidence of stabilizing or recovering. The North Side Pumping Division water levels will continue to decline and recover mostly influenced by climatic changes.

Ground-water mining is caused when discharge exceeds recharge and the decline trend will continue until the resource is exhausted or discharge is reduced to equal recharge. Discharge by pumping from the Snake Plain aquifer presently does not begin to approach recharge. Present net pumping is estimated at about 1.4 million acre-feet per year and recharge is estimated at about 8 million acre-feet per year. Thus, the aquifer can accommodate a substantial increase in ground-water pumping without showing alarming declines.



### Summary and Recommendations

Since construction of the pumping division in the 1950's, well construction methods have changed, especially construction specifications written by Reclamation planners. The original 177 project production wells were drilled by drilling contractors using cable drills, and were completed using the usual completion methods at that time. Drilling was continued below the water table until the drill cuttings were "lost," which was apparently an indication of good yield. Construction completion usually consisted of installing surface casing with the balance of the well left "open hole." When caving conditions were encountered during the drilling, a casing liner was installed, generally just through the caving interval. The liner would be perforated when the caving interval was located within the "good" aquifer section of the well. After the well was completed, a pump test was run to determine the yield. If the yield was insufficient, the well would be deepened in hopes of encountering additional water.

These methods were workable, but generally did not allow for much lowering of the pump if the water level declined. The project was begun about the water level peak period and was completed during a water level decline period. More than one-half of the wells had less than 100 feet of saturated well bore; therefore, as the water levels declined, drawdown increased, the thickness of the saturated well bore thinned, and yield decreased. Deepening of many of the wells was undertaken before the project was completed. About one-half of the wells have been deepened to date (1984) and about one-half of the wells still have less than 100 feet of exposed aquifer.

Under the Recommended Plan, 17 wells (13 new and 4 existing) pumping an average of 14,700 acre-feet annually would provide a full irrigation service to 5,680 acres on eight of the Extension areas (5, 6, 7, 10, 13, 14, 15,

and 17). In addition to providing ground water to the eight Extension areas, 620 acre-feet (annual basis) of ground water will be pumped to 240 acres of square-up land scattered throughout Unit B, and 300 acre-feet (based on limited drainage data) of ground water will be pumped annually to supplement drainage flows to Extension Areas 11 and 12. The ground water pumped to the 240 acres of square-up land (620 acre-feet) and to Area 12 (100 acre-feet) will be provided from existing project wells. A new well will be constructed to provide the supplemental ground-water requirements to Area 11 (estimated at 200 acre-feet per year). This brings the total number of wells under the Recommended Plan to 18.

The plan also provides a replacement water supply for 810 acres of North Side Pumping Division land now irrigated from four project wells located near Extension Area 4 which have inadequate ground-water yields. The replacement water supply for these existing irrigated lands would be provided from the Snake River, and the four existing low yielding wells would be abandoned. This would reduce ground-water pumping by 2,100 acre-feet annually. Thus, the total net annual ground-water pumping requirements for the project would be 13,520 acre-feet annually (14,700 + 620 + 300 - 2,100 = 13,520).

Table 2 is a list of proposed irrigation supply wells and construction specifications for the Extension lands. The depth to water and specific capacity were taken from the previously discussed maps; yearly fluctuations were estimated from well hydrographs and water level data from wells located near each proposed site. An estimated drawdown for each proposed well was based upon specific capacity and design yield. Pump lift is the sum of depth to water, yearly fluctuation, and drawdown and ranges from about 140 feet in tract 13 to about 365 feet in tract 5. The pump suction was placed about 10 feet plus length of the pump below the pump lift or pumping water level.

An additional 50 feet of pump chamber below pump suction was included to allow for pump lowering in case of further water level decline. Thus, the pump chamber includes depth to water, yearly fluctuation, drawdown, 10 feet of pump submergence, length of pump, and 50 feet additional hole. The estimated total depth includes the pump chamber plus additional hole below pump chamber depending upon required yield and well location. Wells with small yield of about 1,000 gallons per minute or slightly more may get by with a total depth near the pump chamber depth. Wells with large yield up to several thousand gallons per minute may require additional hole below the pump chamber.

Maximum total depth may be to 250 feet or more of saturated basalt below the water table. Well diameter will depend upon water volume required at each site. Generally, 20-inch wells are recommended for 2,000 to 3,000 gallons per minute volumes and 24-inch for 4,000 to 5,000 gallons per minute volumes.

During the North Side Pumping Division well drilling in the 1950's, five wells were drilled and not used. These wells are located in tracts 15, 12, 10, and 6 of the Extension lands.

These wells include:

Tract 15-15A724.--A 16-inch diameter, 285-foot-deep well that should yield the required volume as it is constructed.

Tract 12-33A824.--A 20-inch-diameter, 235-foot-deep well that has been sold to the city of Rupert and is not available for use.

Table 2.--Proposed Irrigation Supply Wells and Construction Specifications

		Depth			Yie	eld			Estimate	ed		Well
ract lumber	Well Number	to Water	Yearly Fluctuation	Specific Capacity	Cubic Feet per Second	Gallons per Minute	Pump Length	Drawdown	Pump Chamber	Lift	Total Depth	Casing Diamete
		feet	feet	$gal/min^{1/2}$			feet	feet	feet	feet	feet	inches
5	1	310	10	5,000	8	4,000	10	5	395	325	560	24
-	2	320	10	5,000	8	4,000	10	5	405	335	570	24
	3	290	20	1.000	12	5,500	10	10	340	320	540	28
	4	340	15	1,000	9	4,000	10	10	435	365	590	24
	5	300	15	1,000	5	2,000	5	5	390	320	400	20
	6	300	10	500	4	1,800	5	5	380	315	400	20
	7	300	15	500	5	2,000	5	5	385	320	400	20
	8	310	10	500	3	1,500	5	5	390	325	400	18
6	1	300	10	1,500	7	3,000	5	5	380	315	500	24
-	2	300	10	1,500	6	2,500	5	5	380	315	500	24
7		320	15	500	5	2,500	5	10	410	345	500	24
10	1	240	10	200	5	2,000	5	25	340	275	500	20
	2	230	10	3,000	7	3,500	5	5	310	245	500	24
11		150	10	1,000	2	900	5	5	230	165	230	16
13		120	10	1,000	7	3,500	5	10	205	140	370	24
14		150	10	1,000	2	1,000	5	5	230	165	250	16
15		220	10	2,000	2	900	5	5	300	235	325	16
17		150	15	500	7	3,000	5	10	240	175	400	22

NOTE:

Pump chamber includes:

depth to water

yearly fluctuation of water table

drawdown

length of pump

10-foot submergence below lift

50-foot additional hole for possible future pump lowering

Total depth of well includes:

pump chamber

additional hole depending upon desired yield

 $\pm 1$ / Gallons per minute

Tract 10-25A822.--An 8-inch-diameter, 379-foot-deep well that should have a specific capacity of 100 to 500 gpm/ft which is sufficient, but the well is not capable of yielding 2,000+ gpm because of the small well diameter.

Tract 6-8A921.--A 24-inch-diameter, 383-foot-deep well, and 8B921 an 18-inch-diameter, 376-foot-deep well that should have a specific capacity of about 500 gpm/ft and; therefore, after deepening to about 500 feet, should be capable of yielding the required volume for this tract.

Some or all of these wells may need renovation to bring them up to current Reclamation standards. Renovation of the wells will depend upon the cost being enough less than the cost of a new well to justify the necessary work to bring them up to standard.

Generally, deepening is feasible but reaming to a larger diameter is not.

According to Mr. Virgil Temple, formerly A&B Irrigation District Manager and

now with Reclamation, Water and Land Division, reaming costs about the same per foot as drilling a new well. The high cost is due to problems encountered and extra strain placed upon the drilling equipment.

Well construction should consist of drilling a hole of adequate diameter to the minimum total depth. The total depth can vary somewhat depending upon where the drill site is selected in each tract. The total depth is determined by selecting a depth where the pump can be placed allowing the pumped water level to remain at least 5 feet above the pump bowls after subtracting out drawdown from pumping and natural fluctuations of the water table. Below the pump intake, a pump chamber is drilled about 50 feet into the aquifer. The pump chamber is essentially that portion of the well where the pump is placed and must be deep enough to allow room to lower the pump in case of persistent water level declines. A well in Southern Tract 10 is in an area where the Specific Capacity of the aquifer is about 200 gpm/ft or less and therefore must have an even longer intake area. The deeper penetration of the aquifer utilizes a greater intake area for increased yield by placing the pump suction deeper to permit more drawdown.

The portion of the well deeper than 50 feet below the pump intake may be reduced in diameter. The reduction should decrease drilling costs and will not materially reduce the intake potential. The Johnson Well Screen Company has found that doubling the well diameter only increases yield by about 11 percent.

Casing must be placed in the upper portions of the well to seal out caving zones in the sediment and prevent aquifer pollution from surface waters. The balance of the well can be left open hole; however, for maximum pump protection, casing should be installed throughout the pump chamber.

The local drawdown radius of influence is rarely detectable in the North Side Pumping Division wells even where two wells are near each other.

Therefore, it is not expected to be a problem with the proposed extension wells.

An additional water table decline of 1 or 2 feet may occur from pumping the new wells. Since the overall decline fluctuates with recharge, the actual decline may not be detectable once the wells are established.

Theoretically, the additional pumping for the Extension lands will cause a decrease in the flow at Thousand Springs. Historically, the decrease in flow at the springs has been less than the net withdrawal. Due to the very small increment of additional pumping relative to the existing pumping and because of lag time and attenuation it is unlikely any change in the flow at Thousand Springs can be identified as the effect of the additional pumping.

#### CHAPTER 3--WATER RIGHTS

Any new water rights in the upper Snake River are in question because of the 1982 Idaho Supreme Court ruling. Therefore, a perspective of the so-called Swan Falls controversy is necessary to provide the setting for the water right situation for the North Side Pumping Division Extension.

Idaho Power Company's existing Swan Falls Powerplant is located on the Snake River south of Boise and downstream from the project area. Swan Falls Dam and Powerplant were built in the early 1900's. The Idaho Supreme Court ruled in 1982 that the Swan Falls water rights are not subordinate to later upstream depletions which have been made by irrigation. Under the subordination concept, the power rights would have been inferior to subsequent upstream irrigation water uses. The Supreme Court remanded to the District Court the question as to whether the Idaho Power Company has abandoned or eroded through nonuse some of its Swan Falls water rights.

The lengthy time and costs involved to resolve the Swan Falls issue through litigation may not have necessarily yielded a solution which would have reflected the interests of the participants as well as the public. Therefore, in an attempt to settle the problem, the Idaho Power Company and the State of Idaho identified and mutually agreed upon the following series of judicial, legislative, and administrative actions which should be taken in the public's interest:

- 1. The Snake River minimum streamflow at the Murphy gauge in the State water plan should be adjusted to  $5,600~{\rm ft}^3/{\rm s}$  during the nonirrigation season and  $3,900~{\rm ft}^3/{\rm s}$  during the irrigation season.
- 2. Each new development should be carefully scrutinized against public interest criteria.
- 3. The State should commence a general adjudication of the entire Snake River basin.
  - 4. The State should establish an effective water marketing system.
- 5. State funding should be provided for hydrologic and economic studies to implement the State water plan in the most cost-effective and environmentally sound means.
- 6. Legislation should be enacted to clarify that proceeds from utility sales of hydropower water rights will benefit ratepayers. In March 1985, the required legislation was enacted by the Idaho State Legislature to modify and add the necessary language to Idaho Code which allows full realization of the negotiated contract between the State of Idaho and the Idaho Power Company.

The existing A&B Irrigation District water rights discussed in the "Water Resources" chapter have not been affected by the Swan Falls issue; however, the potential water supplies for the Extension project could be affected by this agreement. The Idaho Department of Water Resources will again begin

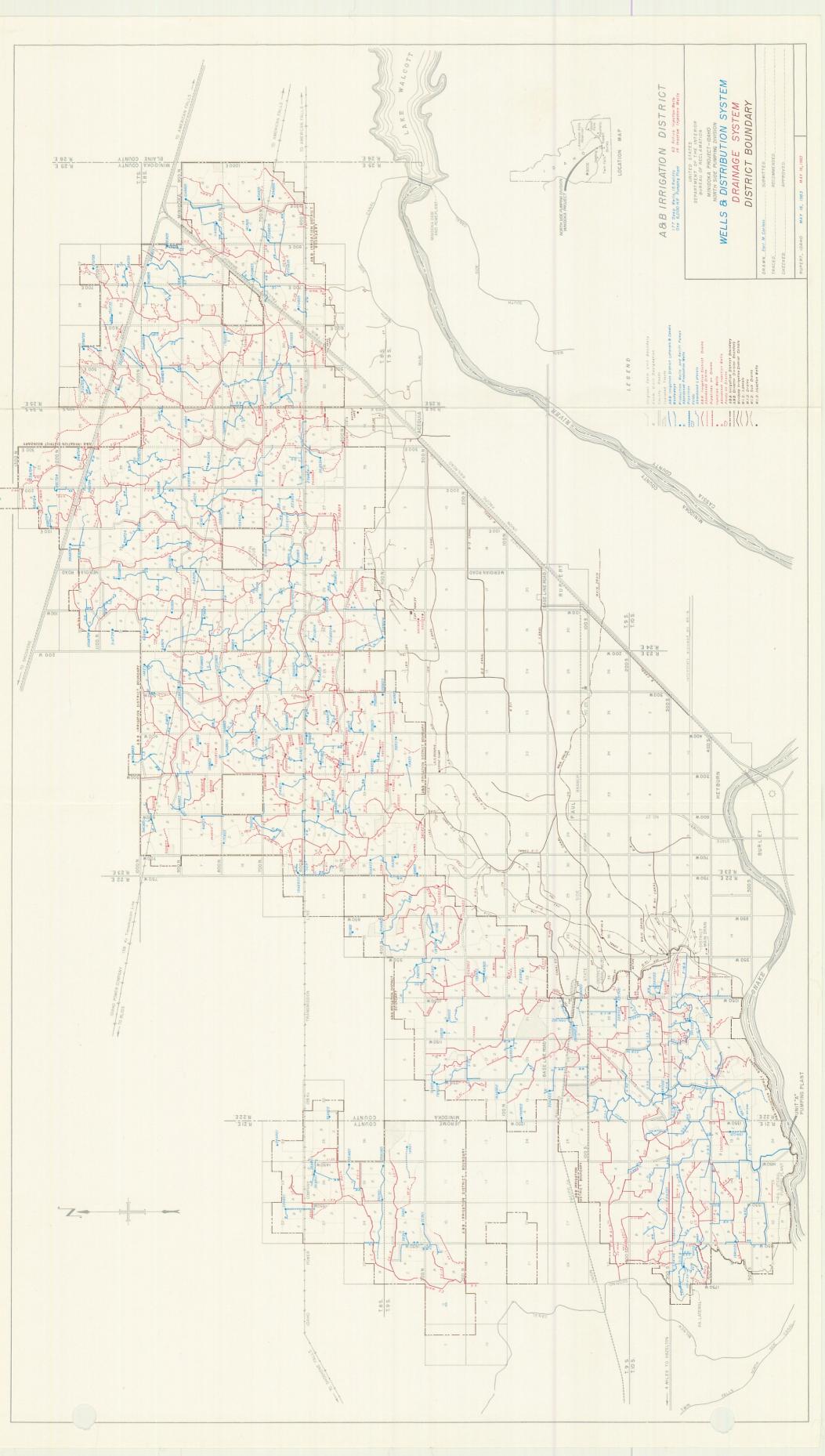
processing water right applications in the not-to-distant future. A ground-water right application (No. 368253) for the new Unit B lands was filed on November 23, 1984, and a surface-water right application (No. 01-7085) for the new Unit A lands was filed on April 22, 1985. The Idaho Department of Water Resources is defining the rules for new applications; Reclamation is working closely with Idaho Department of Water Resources to insure that all proposed water uses are covered by valid State water rights.

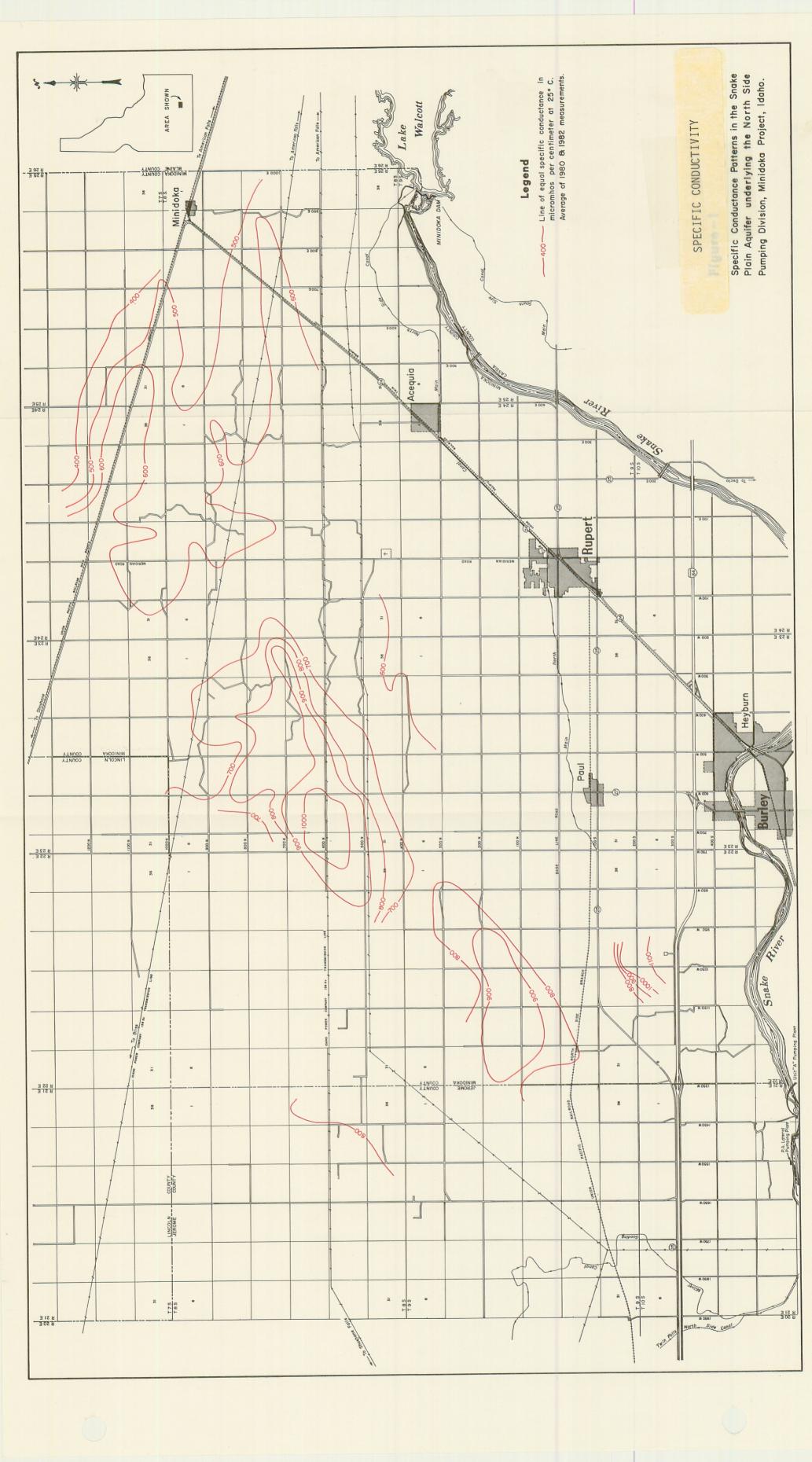
The American Falls and Palisades storage water rights discussed in "Water Resources" chapter were reviewed; about 790 acres of new Unit A lands were not included in the American Falls storage right, and about 1,200 acres of new Unit A lands were not included in the Palisades storage right. However, included in each storage right are Unit B lands served with ground water which do not utilize storage water. To include these new lands in the American Falls and Palisades storage rights, applications to change the place of use were filed with the Idaho Department of Water Resources for each reservoir. These applications transfer the storage water right of part of the Unit B lands discussed above to the new Unit A lands not included in the original filings.

#### CHAPTER 4--WATER QUALITY

Irrigation water supplies for the North Side Pumping Division are withdrawn from the Snake River and from a series of wells on the project.

Most surface drainage is accomplished by the use of injection wells which pass water directly into the underlying aquifer. Project wells, distribution, and drainage systems are shown on the map on the following page.





## Irrigation Water Quality

The quality of water both in the Snake River (table 3) and in ground water pumped from the Snake Plain aquifer (table 4) is good for irrigation purposes. The electrical conductivity of production wells on the North Side Pumping Division was found to range from about 400 micro siemens per centimeter (uS/cm) to slightly above 1,000 uS/cm in a Bureau of Reclamation survey (see Specific Conductivity map). Although yield reductions on sensitive crops such as beans would be expected when irrigated with water having an electrical conductivity greater than 750 uS/cm, beans are generally not grown on areas of the project irrigated with ground water. The low sodium adsorption ratios of surface- and ground-water supplies indicate a low sodium (permeability) hazard. Further, soils in the area have good internal drainage, and no salt problems have occurred after irrigating more than 20 years.

Table 3.--Mean Value and Standard Error of the Mean for Water Quality Parameters from the Snake River near Burley, Idaho (Geological Survey Data)

Parameter	Snake River near Burley	Irrigation Criteria
Electrical conductivity (uS/cm)	445 + 8	<750
$pH^{1/}$ (standard units)	$8.2 \pm 0.1$	4.5-9.0
pH <sup>1</sup> / (standard units) Boron (mg/L) <sup>2</sup> /		<0.75
Calcium (mg/L)	47.0 + 1.0	
Magnesium (mg/L)	$15.6 \pm 0.3$	
Sodium (mg/L)	19.27 + 0.84	
Sodium adsorption ratio (adjusted)	$1.2^{\frac{-3}{4}}$	<3
Chloride (mg/L)	19 + 1	<142
Bicarbonates (mg/L)	190 + 4	
Fecal coliform bacteria (counts		
per $100 \text{ mL} \frac{4}{}$	11 <u>5</u> /	<1,000

<sup>1/</sup> Hydrogen-ion concentration

<sup>2/</sup> Milligram per liter

<sup>3/</sup> Calculated from mean values

<sup>4/</sup> Milliliter

 $<sup>\</sup>overline{5}$ / Geometric mean

Table 4.--Water Quality Characteristics of Representative Production Wells on Unit B, North Side Pumping Division--Chemical Data Were Collected in July 1977, Bacterial Data June 1984

Parameter	32A725	15D823	35D823	118922	Irrigation Criteria
Electrical conductivity (uS/cm)	440	840 <u>3</u> /	583	857 <u>3</u> /	<750 <u>1</u> /
pH (standard units)	7.9	7.8	7.5	7.8	4.5-9.0 <sup>2</sup> /
Boron (mg/L)	0.12	0.15	0.08	0.15	<0.75 <sup>2</sup> /
Calcium (mg/L)	36.9	64.3	64.3	77.2	••
Magnesium (mg/L)	17.3	30.5	19.3	28.7	
Sodium (mg/L)	27.13	59.77	29.66	59.54	
Sodium Adsorption Ratio (adj. SAR)	1.65	$3.60^{3/}$	0.97	2.10	<3 <u>1</u> /
Chloride (mg/L)	34	83	27	57	<142 <u>1</u> /
Carbonates + bicarbonates (mg/L)	161	279	269	353	
Fecal coliform bacteria (counts /100 ml)	<1	<1	<1	<1	<1,000 <u>2</u> /

 $<sup>\</sup>underline{1}/$  From: Ayers, R.S., and R.L. Branson, 1975, Water Quality Guidelines for Interpretation of Water Quality for

# Drainage Water Quality

Most irrigation return flows and storm runoff are disposed of through injection wells which pass water directly into the Snake Plain aquifer. Injected wastewaters occasionally contain sediments and coliform bacteria in excess of Idaho drinking water standards (Graham, Clapp, and Putkey, 1977) and have been linked to contamination of domestic wells in the area. Pesticides, herbicides, and trace metal concentrations in irrigation wastewaters were found to be within drinking water standards (Graham, Clapp, and Putkey, 1977). Careful management is required to prevent and correct domestic water quality problems.

Irrigation return flows entering disposal wells on the North Side Pumping Division do not consistently comply with Idaho standards for injected waters (table 5). Because of the potential for contamination of the Snake Plain

Agriculture. Mimeograph, University of California Coop Extension Service.

2/ From Environmental Protectin Agency, 1972, Water Quality Criteria, Publication No. EPA-R3-73-033, U.S. Government Printing Office, Washington, D.C.

<sup>3/</sup> Problems for sensitive crops such as beans only (not generally cultivated on lands irrigated with ground water); soil permeability unaffected by SAR value

aquifer, Idaho regulations may require some changes in the way project injection wells are operated.

Table 5.--Range of Values for Chemical and Physical Parameters from Water Entering Representative Drain Wells on the North Side Pumping Division

Parameter	22G825	2480824	Well 13AD1021	4BD923	26CD823	Idaho Standard <u>3</u> /
Turbidity (Hach FTU)	2-140	13-260	34-90	2-7	55-300	100
Electrical Conductivity (uS/cm)	487-677	463-635	382-635	697-805	759-853	
pH (standard units)	8.0-8.9	7.4-8.5	7.6-8.6	8.3-8.6	8.0-8.4	
Suspended Solids (mg/L)	5-560	19-692	93-189	7-20	69-1,473	••
Nitrate + Nitrate-N (mg/L)	0.28-0.78	1.14-1.35	0.12-1.50	3.29-3.80	3.24-4.40	10
Total Organic Carbon $(mg/L)^{\frac{1}{2}}$	1.6	12.0		2.2		10
Color (Platinum-Colbalt units) $\frac{2}{}$	<5-5	<5	••	<5-5		100
Odor (TON at 60 degrees $C)^{2/2}$	<5-5	<5-5		<5-5		30
Fecal Coliform Bacteria (counts/100 ml) <sup>2</sup> /	80-330	200-3,300		10-400	••	500
Total Coliform Bacteria (counts/100 ml)2/	700-1400	7000		1000-6000		10,000

The injection well problem on the A&B Irrigation District was not addressed in the Extension study because of the short time frame, except that reuse of existing drain flows was incorporated into the proposed plan wherever feasible.

Considerable work has already been done by the State of Idaho (1983) and the Bureau of Reclamation on alternatives to the injection wells. Additional study efforts by Reclamation and A&B Irrigation District to gather necessary data and to develop a feasible solution to the injection well problem will continue concurrently with and beyond the Extension study.

<sup>1/</sup> Results from single sample, July 1984  $\overline{2}/$  Results from 2 samples collected in June and July 1984  $\overline{3}/$  Existing drain wells

### Impacts of Proposed Plan

The proposed Extension plan would involve a net pumpage of about 13,520 acre-feet from the Snake Plain aquifer and  $12,300^{1/2}$  acre-feet from the Snake River. Present use of the Snake Plain aquifer for irrigation includes approximately 210,000 acre-feet per year on the North Side Pumping Division and an estimated total of 600,000 acre-feet per year on the entire lower Snake Plain area between Rupert and Thousand Springs, Idaho. Current average annual irrigation diversions from the upper Snake River system are estimated to be 8.2 million acre-feet per year.

Since all new Extension lands will be irrigated with sprinklers, return flows will be minimized and held in ponds for reuse or loss through evaporation and deep percolation. Although some increases in dissolved salts and the resultant electrical conductivity of the Snake Plain aquifer and Snake River could occur through deep percolation of return flows, this would be a slight impact, if measurable, because return flows from Extension lands would be a minor source of water and salts to both the aquifer and river. Since no return flows from new irrigation will be disposed of in injection wells, Extension lands would not contribute to bacterial and sediment problems in the Snake Plain aquifer.

Potential for reuse of existing irrigation return flows was examined under the Extension study as a partial solution to the injection well problem on the North Side Pumping Division. Reuse of drainwater from H Main Drain, (now entering drain wells 4BD923 and 5AD823, see A&B Irrigation District map) E Main Drain, and the F Main Drain (now entering drain wells 9AD824, 11AD823,

<sup>1/</sup> The proposed surface-water pumping of 12,300 acre-feet includes 2,500 acre-feet for 810 acres of land presently served with ground water that would be served with Snake River water.

32AD824, and Cap Hawley) were found feasible; however, a new irrigation well would be required to augment the H Main Drain flows for much of the irrigation season, and existing project wells would be required to augment both early and late season drainwater flows in the E and F Main Drains. The records maintained on the drains and injection wells in the A&B Irrigation District indicate there is about 100 acre-feet per year available to the new lands at the terminus of H Main Drain (Area 11) and about 1,200 acre-feet per year available to the new land between E and F Main Drains (Area 12).

The quality of drainwaters proposed for reuse is suitable for irrigation (table 6). Reuse should produce a minor reduction in bacterial and sediment contamination of the Snake Plain aquifer.

Table 6.--Water Quality Characteristics of Drainage Water Proposed for Reuse under the North Side Pumping Division Extension Study--Data Were collected in September 1984

	"H Main"	<del></del>	"F Main"	A /	Irrigation
Parameter	4BD923	32AD824	Cap Hawley	9AD824 <sup>4</sup> /	Criteria
Electrical conductivity (uS/cm)	785	756	646	632	<750 <u>1</u> /
pH (standard units)	8.4	8.3	8.0	8.0	4.5-9.0 <u>2</u> /
Boron (mg/L)	0.33	0.55	0.58		<0.75 <sup>2</sup> /
Calcium (mg/L)	60.5	61.5	55.9		
Magnesium (mg/L)	31.4	27.5	23.5		
Sodium (mg/L)	64.14	57.48	45.75		
Sodium Adsorption Ratio (adjusted)	3.58	3.30	2.79		<3 <u>1</u>
Chloride (mg/L)	79	80	62		<142 <u>1</u>
Carbonates + bicarbonates (mg/L)	265	248	227		
Fecal coliform bacteria (counts/100 ml)	30	400	600		4,000 <sup>2</sup>

<sup>1/</sup> From: Ayers, R.S. and R.L. Branson, 1975, Water Quality Guidelines for Interpretation of Water Quality for Agriculture. Mimeograph, University of California Coop Extension Service.

4/ Data collected in September 1983.

<sup>2/</sup> From: Environmental Protection Agency, 1972, Water Quality Criteria, Publication No. EPA-R3-73-033, U.S. Government Printing Office, Washington, D.C.

<sup>3/</sup> Problems for sensitive crops, such as beans only (not generally cultivated on lands irrigated with ground water); soil permeability unaffected by Sodium Adsorption Ratio.

The Environmental Protection Agency has received a petition requesting designation of the Snake Plain aquifer as a sole source of drinking water. If the Snake Plain aquifer is designated a sole source of drinking water, Environmental Protection Agency would be required to review and approve projects which might contaminate the aquifer. It seems unlikely that sole source review would result in any changes in the proposed Extension plan because (1) no drainage water from Extension lands would be disposed of in existing drain wells, (2) no new drain well would be drilled to dispose of drainwater, and (3) some water now entering injection wells would be applied to new lands.

## Future without Proposed Plan

The quality of water pumped from the Snake River and the Snake Plain aquifer to serve North Side Pumping Division lands is suitable for presently irrigated crops and is expected to remain suitable in the future. No major changes in the concentration of inorganic salts are expected in water supplied from either the Snake River or the Snake Plain aquifer since little additional irrigation development is projected in areas upgradient of the North Side Pumping Division.

Most surface drainage on North Side Pumping Division lands is accomplished through the use of injection wells which pass water directly into the Snake Plain aquifer. Irrigation return flows generally contain fecal coliform bacteria in excess of Idaho drinking water standards and have been linked to contamination of domestic wells in the area. Bacterial contamination of domestic wells in the Snake Plain aquifer is expected to decline in the future as problem disposal wells are identified, pollution sources cleaned up, and alternatives to present wastewater injection practices implemented as required by Idaho regulations governing use of disposal wells.

If the Snake Plain aquifer were designated as a sole source of drinking water, this designation is not expected to impact operation of existing irrigation and drainage facilities on the North Side Pumping Division because sole source review is not retroactive.

#### CHAPTER 5--IRRIGATION WATER REQUIREMENTS

## Summary of Results

Average monthly diversion requirements and peak delivery requirements were estimated for the period 1965 to 1982 using representative climatological data. It was assumed all newly irrigated lands would be served by sprinkler systems having an irrigation efficiency of 70 percent. The delivery system serving lands in Unit A differs from the system serving lands in Unit B; therefore, separate water requirements were derived for each unit.

# Monthly Diversion Requirements

The average monthly and the average annual diversion requirements are presented in tables 7 and 8, respectively. These diversion requirements are adopted for use in project planning studies.

Table 7.--Average Monthly Diversion Requirements

		Diversion Requirements				
Month	Distribution	Unit A	Unit B			
	percent	inches pe	racre			
April	1.7	0.65	0.54			
May	8.7	3.25	2.71			
June	25.9	9.67	8.04			
July	36.3	13.55	11.27			
August	17.7	6.62	5.51			
September	7.5	2.82	2.35			
October	2.2	0.81	0.67			
Total	100.0	37.37	31.09			
		(3.11 feet)	(2.59 feet			

Table 8.--Annual Diversion Requirements

Source	Area	Annual Diversion Requirement
	acres	acre-feet
Snake River supplies from North Side		
Pumping Division	3,950	12,300
Drainwater from existing North Side	580	1,500 <u>1</u> /
Pumping Division drains  Ground water—		1,500-
Ground water=/	<u>5,680</u>	14,700
Total	10,210	28,500

<sup>1/</sup> Drainwater would need to be augmented with ground-water pumping. For Area 12, existing project wells would be required to augment both early and late season drainwater flows. A new well would be required to augment H Main Drain flows for much of the irrigation season to serve lands in Area 11.

## Design Peak Delivery Rate

A peak farm delivery rate of 0.434 inch per day was estimated during the course of this study. This rate is well within acceptable limits; however, the ABID views the development of the Extension lands as a completion of the original project. In a letter to the Bureau of Reclamation dated May 24, 1984, the district states that they cannot support a peak net farm delivery in excess of 0.357 inch per day, which is the rate at which the current project is designed and operated. Therefore, a peak farm delivery of 0.357 inch per day was adopted for use in this study. This rate represents the capacity at the turnouts. Additional capacity is required for conveyance losses in the laterals and canals.

#### General

The irrigation water requirements are discussed in two sections. In the first section, monthly diversion requirements are computed based on average

<sup>2/</sup> Does not include 240 acres of square-up lands scattered throughout Unit B. The annual diversion requirements for this land is 620 acre-feet and will be provided from existing project wells in Unit B.

monthly consumptive use (evapotranspiration) estimates. In the second section, a peak delivery requirement is estimated and used to develop pump, canal, and lateral sizing criteria.

All consumptive use estimates (monthly and peak) are accomplished using the Jensen-Haise procedure assuming a mean project elevation of 4250 feet.

The Jensen-Haise equation for estimating reference evapotranspiration is:

$$E_{tr} = C_t (T - T_x) R_s$$

where: E<sub>tr</sub> is reference ET based on alfalfa with 30 to 50 centimeters of top growth, inches per day.

T is mean daily temperature, °F; T is the temperature axis intercept, °F;  $T_x^x = 27.5 - 0.25 (e_2 - e_1) - (elevation/1000).$ 

R<sub>s</sub> is global solar radiation, calories per square centimeter per day (langleys/day), (multiplied by 0.000673 to obtain inches per day).

 $C_T$  is an empirical coefficient;

$$C_t = 1/(C_1 + C_2 C_H)$$
 $C_1 = 68 - (3.6 \times elevation/1000)$ 
 $C_2 = 13$ 
 $C_H = 50/(e_2 - e_1)$ 

 $e_2$  and  $e_1$  are saturation vapor pressure (e in millibar) of water at the long-term (normal) mean maximum and mean minimum temperatures, respectively, for the warmest month (usually July) of the year for the study area.

The term "reference ET" has replaced "potential ET;" however, the computer output labeling has not been updated. Therefore, in this study, the two terms should be considered synonymous.

## Section I -- Monthly Diversion Requirement

Diversion requirements were derived by first determining the average monthly consumptive use of the crops projected to be grown. The basis of the

Jensen-Haise procedure is that consumptive use is a function of temperature and solar radiation given an adequate water supply. The effective precipitation is then subtracted from the estimated consumptive use to obtain the water needed to be supplied by irrigation. A farm irrigation efficiency is then applied to the crop irrigation requirement to obtain the farm delivery requirement. Losses associated with this efficiency are assumed to occur because of wind drift, application evaporation, runoff, and deep percolation. Expected seepage losses from conveyance systems along with operational spills are added to the farm delivery requirement to obtain the diversion requirement.

Consumptive Use. -- The computer program XCIR was employed to aid in the computation of the monthly crop irrigation requirements. The program first computes reference evapotranspiration ( $E_{\rm tr}$ ) from monthly values of solar radiation and temperature and then calculates crop evapotranspiration (ET) by multiplying the  $E_{\rm tr}$  by an appropriate crop coefficient. The program also computes that portion of precipitation considered available to satisfy crop evapotranspiration requirements. Effective precipitation is a function of the total precipitation, rainfall intensity, infiltration rate of the soil, water holding capacity of the soil, and the evapotranspiration rate. The crop irrigation requirement is then computed as crop ET less the effective precipitation and any nongrowing season soil moisture carryover. Any excess soil moisture is considered runoff or deep percolation.

Data Input to XCIR.--Input data to the XCIR program includes average monthly temperature, monthly precipitation, average monthly solar radiation, maximum water holding capacity, and the projected crop distribution for the project area. Other input included the long-term average maximum and minimum temperatures in July, the growing seasons and cover dates for each crop, and crop coefficients.

Temperature and Precipitation. --Daily temperature and precipitation data have been collected continuously from May through September on the Minidoka North Side Project as part of an Irrigation Management Service (IMS) program. To utilize this data, a substantial number of temperature and precipitation values would have to be estimated for April and October because the growing season for the majority of the crops in this study begins in April, with approximately 50 percent of the crops growing into the month of October. Further, this data does not lend itself to utilizing the effective precipitation and carryover soil moisture routine in XCIR because this routine requires average monthly climatological data for the entire year. For these reasons an alternate weather station was selected to represent the Minidoka North Side Project.

The Twin Falls Weather Service Office (WSO) has collected daily temperature and precipitation data on a long-term basis. This station is located at the Idaho Water and Energy Resources Research Institute (Snake River Conservation Research Station, U.S. Department of Agriculture-Agricultural Research Service), Kimberly, Idaho, approximately 36 miles southwest of the project area. Although other stations were closer to the project area, this weather station was selected primarily because of its surrounding agricultural environment. Further, it is the nearest station which collects daily solar radiation throughout the year.

The Twin Falls average monthly temperature and precipitation data for a 17-year period (1965-72 and 1974-82) was compared to the IMS average monthly climatological data for the May through September period. As shown in table 9, the average monthly temperatures at Twin Falls are approximately 1 °F higher than the average monthly IMS temperatures. The average monthly

precipitation data compares quite favorably with the only exception occurring in September. In this case, the Twin Falls precipitation is slightly higher.

Table 9.--Comparison of Average Monthly Temperature and Precipitation, 1965-72 and 1974-82

Station	Climatic Variable	May	June	July	August	September $\frac{1}{}$
IMS	Temperature (°F)	52.9	61.1	68.0	66.3	57.3
	Precipitation (inches)	1.07	0.79	0.31	0.57	0.61
Twin Falls	Temperature (°F)	53.7	62.1	69.1	67.3	58.2
(WSO)	Precipitation (inches)	1.03	0.78	0.33	0.53	0.83

<sup>1/</sup> Represents the period 1965-72 and 1978-82

The consistent difference in temperature (1 °F) may be explained by the following reasons: first, the differences in the calibration of the temperature recording devices and second, the elevation of the Twin Falls station is 3960 feet which is 250 feet lower than the IMS station.

Three additional stations (adjacent to the project) were selected for annual temperature comparisons to assure the Twin Falls station was representative of the project area. They are Burley Federal Aviation Administration (FAA) at elevation 4157 feet; Minidoka Dam at elevation 4210 feet; and Paul at elevation 4210 feet. The average annual temperatures are compared for each station from 1965 through 1982 exclusive of 1975-76 period (Minidoka Dam has missing data in this period) in table 10.

Table 10.--Comparison of Average Annual Temperatures, 1965-74 and 1977-82

	Twin Falls	Burley FAA	Minidoka Dam	Pau 1
Temperature (°F)	47.7	47.8	48.1	47.1

There are no significant temperature differences among the four stations; therefore, it was assumed the Twin Falls station was representative of the project area and its data was used in the consumptive use computations. The period selected for computation was restricted to 1965 through 1982 due to the availability of the solar radiation data at the Twin Falls station. The monthly temperature data and precipitation data are summarized in tables 11 and 12, respectively.

Table 11.--Monthly Average Temperature in Degrees (Estimate), Temperature Data, Twin Falls WSO Elevation = 3960 Feet

	TEMPERAT	TURE DA	TA,TWIN	FALLS	SO ELEV	ATION=	3960 FT	٠.				
YEAR	JAN.	FEB.	MARCH	APRIL	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.
1965	31.3	34.0	34.9	48.9	51.2	60.5	68.1	65.9	53.4	51.9	42.6	29.5
1966	29.4	30.5	38.8	46.1	58.5	62.1	70.2	67.8	62.7	46.9	40.6	29.3
1967	33.0	36.2	39.8	41.3	54.1	60.2	72.3	71.9	63.4	47.5	39.6	24.8
1968	27.5	36.3	42.0	43.3	54.1	62.8	70.7	62.6	57.1	47.4	37.3	28.6
1969	31.2	30.0	36.1	48.4	58.9	61.8	69.4	69.2	61.5	43.3	38.5	32.2
1970	32.7	38.0	38.3	40.6	53.8	62.7	69.6	69.9	53.5	44.8	39.1	28.2
1971	30.8	33.1	36.4	44.1	54.9	60.8	69.4	71.3	53.5	44.9	37.0	25.7
1972	26.0	31.4	42.7	44.6	54.8	63.6	67.3	68.6	54.6	47.8	36.1	23.5
1973	26.0	32.6	38.2	44.7	56.3	62.4	68.5	67.6	56.9	49.1	37.5	33.3
1974	24.9	33.6	38.2	45.3	53.4	66.2	68.9	65.8	59.5	48.5	38.5	30.2
1975	24.8	30.3	37.6	39.4	50.2	60.6	72.4	64.0	59.2	47.9	35.6	33.8
1976	28.8	29.6	33.4	44.7	55.9	59.7	69.3	63.4	59.6	47.4	39.3	29.9
1977	23.0	33.8	36.8	49.8	50.1	66.6	68.1	67.9	57.4	49.5	37.2	34.9
1978	33.6	34.3	43.8	45.9	51.4	61.0	67.8	64.2	56.9	49.7	34.3	25.1
1979	16.1	31.7	40.1	45.0	55.5	62.7	68.6	67.2	62.9	51.6	32.8	32.0
1980	26.1	36.6	38.0	48.9	53.1	60.4	68.9	64.7	59.0	48.3	37.7	33.1
1981	31.8	33.9	41.2	47.9	52.9	61.7	67.8	70.0	60.1	45.4	40.4	33.5
1982	23.3	26.2	39.2	42.0	52.0	61.4	66.6	69.0	58.2	46.1	34.2	29.5
MEANS	27.8	32.9	38.6	45.0	53.9	62.1	69.1	67.3	58.3	47.7	37.7	29.8

Table 12.--Monthly Precipitation, Twin Falls WSO

PRECIP	NOITATION	IN INCH	ES		•	VALUES B	ASED ON	ESTIMATE	) TEMP A	ND OR PR	ECIP		
YEAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	ост	NOV	DEC	TOTA
1965	. 79	. 21	. 18	2.08	1.14	. 5 1	. 38	. 87	. 60	0.00	.74	. 79	8.29:
1966	. 36	. 22	. 85	. 26	. 33	. 10	. 33	.02	. 16	. 28	1.15	. 56	4.62:
1967	. 80	. 10	1.20	1.90	. 58	2.38	. 29	.01	. 28	. 94	. 36	. 17	9.01:
1968	. 76	1.68	1.00	. 44	. 93	. 93	0.00	3.23	. 29	. 31	1.62	1.97	13.16:
1969	. 5 1	. 76	.08	. 27	. 12	1.01	. 16	0.00	. 85	. 43	. 68	1.23	6.10:
1970	3.38	. 19	. 57	. 75	1.10	1.62	1.09	. 05	1.03	. 58	2.32	1.09	13.77:
1971	2.09	. 31	. 95	2.04	2.00	1.40	.01	. 29	. 75	.71	1.90	1.05	13.50
1972	2.81	1.18	1.14	. 35	. 32	. 90	.01	. 32	. 92	1.32	.89	1.36	11.52:
1973	. 97	. 38	1.84	1.00	. 60	1.07	. 50	. 07	. 89	. 26	2.10	1.78	11.46:
1974	. 75	. 77	1.91	. 70	15	. 25	. 27	. 08	0.00	1.19	. 37	1.31	7.75
1975	. 84	1.71	2.18	1.44	2.79	. 37	. 45	. 06	. 02	2.59	. 86	. 49	13.80
1976	1.52	1.29	. 43	. 87	. 41	. 8 1	. 26	2.02	. 79	.72	. 10	0.00	9.22
1977	. 34	. 22	. 18	. 14	1.86	. 72	1.27	. 36	.78	.04	1.77	2.03	9.71
1978	1.32	. 83	1.72	1.40	1.19	. 14	. 12	. 22	2.41	.01	. 52	. 30	10.18
1979	2.28	. 49	1.13	. 32	. 73	. 46	. 11	. 78	. 02	. 83	. 82	. 32	8.29
1980	2.71	. 88	. 34	. 88	2.74	. 99	. 18	. 11	1.73	. 34	. 77	1.04	12.71
1981	. 51	. 44	. 45	1.92	. 76	. 28	.01	0.00	. 35	1.21	1.72	2.95	10.60
1982	. 7 <b>7</b>	1.08	1.38	. 94	. 39	. 52	. 70	. 43	1.48	1.60	. 76	1.25	11.30
<b>A</b> VE	1.31	. 71	. 97	. 98	1.01	. 80	. 34	. 50	.74	.74	1.08	1.09	10.28

Additional temperature input required for XCIR program is the average daily maximum and minimum temperatures for the warmest month. These average daily temperatures are utilized in the program to compute the temperature coefficient ( $C_t$ ) and the temperature intercept ( $T_x$ ). The warmest month for this area generally occurs in July, and the average daily maximum and minimum July temperatures for the 18-year period (1965-82) are 85.0 °F and 53.2 °F, respectively.

Solar Radiation.--Daily solar radiation data was also collected on the Minidoka North Side Project as part of the IMS program. For the reasons discussed in the temperature and precipitation section above, an alternate solar radiation station was selected to represent the project area.

The Twin Falls weather station average monthly solar radiation data is available from 1965 through 1982. As shown in table 13, the Twin Falls average monthly solar radiation data is very close to the average monthly IMS solar radiation (the period selected for comparison represents the available IMS data). Therefore, it was assumed the Twin Falls data was

representative of the project area and was adopted for use in the consumptive use computations.

Table 13.--Comparison of Solar Radiation Data

		Monthly	Average1	965 <b>-</b> 72 and 1	974-82
Station	May	June	July	August	September 1/
			langleys p	er day	
Minidoka IMS	548	616	632	549	434
Twin Falls WSO	566	620	636	545	428

<sup>1/</sup> September represents 1965-72 and 1978-82

The average monthly solar radiation from 1965 through 1982 is presented in table 14.

Table 14.--Monthly Average Solar Radiation in Langleys, Solar Radiation, Twin Falls WSO Elevation = 3960 Feet

	SOLAR	RADIATIO	N, TWIN	FALLS	WSO ELE	=NOITAV	3960 F	Γ.				
YEAR	JAN.	FEB.	MARCH	APRIL	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.
1965	143.	290.	436.	464.	536.	607.	628.	537.	449.	352.	189.	149.
1966	159.	243.	384.	520.	620.	634.	663.	571.	431.	323.	191.	152.
1967	135.	278.	341.	404.	596.	562.	618.	579.	426.	315.	178.	132.
1968	175.	242.	373.	50 <b>6</b> .	590.	622.	675.	467.	454.	322.	185.	135.
1969	157.	235.	407.	527.	625.	568.	657.	574.	435.	297.	213.	116.
1970	136.		336.	493.	584.	596.	618.	557.	438.	310.	184.	138.
1971	159.	248.	356.	418.	557.	620.	661.	540.	446.	297.	168.	153.
1972	182.		369.	477.	571.	627.	668.	540.	406.	277.	158.	160.
1973	163.		357.	499.	631.	649.	635.	560.	445.	314.	147.	106.
1974	153.		344.	515.	637.	708.	658.	603.	518.	299.	188.	125.
1975	194.		316.	392.	540.	621.	611.	571.	492.	284.	197.	145.
1976	166.		391.	465.	611.	667.	634.	522.	441.	334.	227.	175.
1977	149.		334.	521.	468.	603.	616.	487.	414.	310.	167.	114.
1978	138.	197.	338.	391.	540.	642.	621.	561.	399.	334.	165.	145.
1979	175.		352.	463.	573.	639.	618.	469.	453.	279.	200.	134
1980	150.	215.	319.	434.	478.	557.	573.	519.	372.	306.	184.	138
1981	159.	248.	356.	466.	505.	646.	697.	606.	471.	254.	184.	138
1982	173.		303.	479.	595.	628.	603.	566.	390.	292.	185.	129
MEANS	159.	248.	356.	469.	570.	622.	636.	546.	438.	306.	184.	138

Crop Distribution and Growing Season. -- The project crop distribution of the Minidoka North Side Extension is shown in table 15. The pattern is primarily based on historical cropping patterns of the A&B Irrigation District

and the Minidoka Irrigation District. The current historical pattern of the A&B Irrigation District would have been used except the Extension lands are expected to have more forages and less grains than the existing A&B Irrigation District cropping pattern.

Table 15.--Cropping Pattern

Crop	Percentage
Barley (spring) Wheat (2/3 spring, 1/3 winter) Dry beans Sugar beets Alfalfa hay	15 24 8 26 27
Total	100

Key planting, cover, and harvesting dates for the most prevalent crops grown near Kimberly, Idaho, are published by the Bureau of Reclamation in the "Technical Guideline for Estimating Agricultural Crop Water Requirements" dated September 1983. In 1970, the Soil Conservation Service (SCS) published an "Irrigation Guide for Southern and Southeastern Idaho" containing planting and harvesting dates for 17 crops grown in the Rupert, Idaho, area. These planting and harvest dates in the SCS guide for the above five crops compared favorably with the respective Reclamation technical guideline dates. The SCS irrigation guidelacks cover data information; therefore, the Reclamation irrigation guideline data was selected for the consumptive use computations. The appropriate growth stage data used in the XCIR program is illustrated in table 16.

Table 16.--Growth Stage Data

	j¥	lonth/Day	y	Time (Days)		
Crop		Full			Full Cover	
		Cover	Harvest	Full Cover	to Harvest	
	4/04	6/20	8/10	80	55	
ŕ	$\frac{2/15}{(10/10)^{1/2}}$	6/05	8/10	110	60	
	5/22	7/15	8/30	55	45	
	4/15	7/10	10/15	85	95	
1st	4/01	5/01	6/15		76	
2nd	6/15	,	8/01		46	
3rd	8/01		9/15		46	
4th	9/15		10/30		46	
	at)  1st 2nd 3rd	at) $4/04$ 2/15 (10/10) $1/5/224/151st 4/012nd 6/153rd 8/01$	Planting Cover  at) 4/04 6/20 2/15 1/6/05 (10/10)1/5/22 7/15 4/15 7/10  1st 4/01 5/01 2nd 6/15 3rd 8/01	Planting Cover Harvest  at) 4/04 6/20 8/10 2/15 / 6/05 8/10 (10/10) 1/ 5/22 7/15 8/30 4/15 7/10 10/15  1st 4/01 5/01 6/15 2nd 6/15 8/01 3rd 8/01 9/15	Planting Cover Harvest Full Cover  at) 4/04 6/20 8/10 80  2/15 6/05 8/10 110  (10/10)1/ 5/22 7/15 8/30 55 4/15 7/10 10/15 85  1st 4/01 5/01 6/15 2nd 6/15 8/01 3rd 8/01 9/15	

<sup>1/</sup> Effective planting data used for computations; effective germination is spring green-up, actual dates in parentheses.

Soil Moisture.--A research program on "Use of Water on Federal Irrigation Projects" was conducted by Reclamation on the Minidoka North Side Pumping Division from 1964 through 1968. Soil moisture data was collected as part of this program for selected crops grown in the project area. This data is inclusive of the five crops in the proposed crop distribution and is summarized for each crop and the weighted crop distribution in table 17. The available soil moisture is defined as the amount of moisture available to the plant between field capacity (one-third atmosphere) and permanent wilting point (15 atmospheres). The usable soil moisture is the amount of soil moisture depletion at which optimum crop yields can be maintained.

<sup>2/</sup> Effective planting data for established alfalfa is data growth begins in spring or at harvest of preceding crop; final harvest is data crop becomes dormant.

Table 17.--Soil Moisture

Crop	Effective Root Zone	Available Moisture	Usable Moisture	
	feet	inches	inches	
Spring grains	3.0	7.8	3.9	
Winter wheat	4.0	10.0	4.5	
Dry beans	2.0	5.2	2.5	
Sugar beets	3.0	7.8	3.5	
Alfalfa	5.0	12.0	6.0	
Weight crop distribution	3.5	8.9	4.3	

In computating effective precipitation and accounting for soil moisture, the XCIR program requires maximum available water and an initial soil moisture content. The maximum available water for the crop distribution is 8.9 inches, and it was assumed for this study the initial soil moisture content was 4.3 inches.

<u>Crop Coefficients</u>.--Information for accessing computer programs and data files pertaining to evapotranspiration computations are included in the Reclamation "Technical Guideline for Estimating Agricultural Crop Water Requirements" (dated September 1983). One of these data files (CRPCOFM) contains crop coefficient data (based on data developed by Dr. Marvin Jensen, et al., in 1973) which is specifically formatted for use with the XCIR program. These crop coefficients were assumed applicable to the project area and were used in the monthly consumptive use computations.

Further, the computer program (PEAKWRN) used to compute the peak daily ET includes a subroutine with crop coefficient data. This data was compared with the respective coefficient data in the above file (CRPCOFM) for the five crops proposed to be grown, and no differences were noted.

XCIR Results and Comparison with Other Estimates.--The XCIR results consist of estimated monthly reference (potential) ET, estimated monthly ET

for each crop and the weighted crop distribution, and the estimated crop irrigation requirements for the weighted crop distribution.

A monthly summary of the weighted crop evapotranspiration (ET) for the period 1965 through 1982 is presented in table 18. The average annual weighted crop irrigation requirements (ET minus effective precipitation) for the 18-year period is 20.68 inches per acre. Monthly and annual tabulations of the crop irrigation requirement are found in table 19.

The computed average annual weighted crop irrigation requirement was compared with data in two publications, the SCS "Irrigation Guide for Southern and Southeastern Idaho" (dated 1970) and Bulletin 516, "Consumptive Irrigation Requirements for Crops in Idaho," published by the University of Idaho in 1970. In both publications, crop irrigation requirements are developed for Rupert, Idaho. As shown in table 20, the computed annual weighted crop irrigation requirement compares reasonably with the data contained in the above two publications.

Table 18.--Monthly Evapotranspiration in Inch, Minidoka North Side Extension Weighted Crops

EAR	JAN.	FEB.	MARCH	APRIL	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.	TOTAL	
1965	0.00	.01	.04	1.13	3.07	5.74	7.56	3.90	2.00	1.26	0.00	0.00	24.71	
1966	0.00	.01	.05	1.16	4.30	6.21	8.30	4.31	2.40	1.00	0.00	0.00	27.74	
1967	0.00	.01	.04	. 75	3.70	5.28	8.05	4.72	2.41	. 99	0.00	0.00	25.95	
1968	0.00	.01	.05	1.02	3.67	6.19	8.53	3.17	2.23	1.01	0.00	0.00	25.88	
1969	0.00	0.00	.04	1.26	4.38	5.53	8.11	4.45	2.36		0.00	0.00	26.94	
1970	0.00	.01	.04	.89	3.60	5.92	7.65	4.37	1.96	. 89	0.00	0.00	25.33	
1971	0.00	.01	.04	. 87	3.53	5.90	8.16	4.35	1.99	. 86	0.00	0.00	25.71	
1972	0.00	.01	. 05	1.01	3.61	6.34	7.92	4.14	1.87	. 88	0.00	0.00	25.83	
1973	0.00	.01	. 04	1.06	4.15	6.40	7.70	4.21	2.17	1.04	0.00	0.00	26.78	
1974	0.00	.01	. 04	1.11	3.88	7.56	8.05	4.37	2.69	. 97	0.00	0.00	28.68	
1975	0.00	0.00	. 04	. 67	3.01	5.89	7.97	3.99	2.54	. 91	0.00	0.00	25.02	
1976	0.00	.01	. 03	. 98	3.98	6.19	7.81	3.60	2.29	1.05	0.00	0.00	25.94	
1977	0.00	.01	. 04	1.30	2.60	6.49	7.41	3.68	2.04	1.04	0.00	0.00	24.61	
1978	0.00	.01	. 05	. 86	3.12	6.14	7.43	3.94	1.95	1.12	0.00	0.00	24.62	
1979	0.00	0.00	. 04	. 99	3.69	6.34	7.51	3.50	2.54	. 99	0.00	0.00	25.60	
1980	0.00	.01	.04	1.05	2.89	5.25	7.00	3.68	1.91	. 99	0.00	0.00	22.82	
1981	0.00	.01	. 05	1.10	3.04	6.28	8.34	4.77	2.48	. 75	0.00	0.00	26.82	
1982	0.00	0.00	. 04	. 92	3.49	6.06	7.05	4.37	1.96	. 88	.0.00	0.00	24.77	
MEANS	0.00	0.00	.04	1.01	3.54	6.10	7.81	4.08	2.21	. 97	0.00	0.00	25.76	25.7
MAX	0.00	. 01	. 05	1.30	4.38	7.56	8.53	4.77	2.69	1.26	0.00	0.00		
MIN	0.00	0.00	.03	. 67	2.60	5.25	7.00	3.17	1.87	. 75	0.00	0.00		
STDEV	0.00	.00	.01	. 16	. 50	. 52	. 43	. 43	. 26	. 12	0.00	0.00		
ĸc	0.00	.01	. 02	. 28	. 60	. 8 1	. 85	54'	. 46	. 37	0.00	0.00		

Table 19.--Monthly Crop Irrigation Requirements, Minidoka North Side Extension, Elevation = 4250 Feet

		C END-OF TER HOLD		100 1 CITY = F			90 90	UES BASE 90 90	90 90				AWC AT FIELD CAP
EAR	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
1965	0.00	0.00	0.00	0.00	1.16	5.28	7.22	3.12	1.46	1.26	0.00	0.00	19.50:
1966	0.00	0.00	0.00	. 93	3.11	6.12	8.00	4.29	2.26	. 75	0.00	0.00	25.46:
1967	0.00	0.00	0.00	0.00	2.29	3.25	7.79	4.71	2.16	. 14	0.00	0.00	20.34:
1968	0.00	0.00	0.00	. 62	1.94	5.35	8.53	. 55	1.97	. 73	0.00	0.00	19.71:
1969 1970	0.00	0.00	0.00	1.02	3.38	4.62	7.97	4.45	1.60	. 42	0.00	0.00	23.45:
	0.00	0.00	0.00		1.73	4.49	6.67	4.33	1.03	. 37	0.00	0.00	18.83:
1971	0.00	0.00	0.00	0.00	89	4.66	8.15	4.09	1.32	. 22	0.00	0.00	19.33:
1972	0.00	0.00	0.00	. 70	2.43	5.53	7.91	3.85	1.04	0.00	0.00	0.00	21.46:
1973	0.00	0.00	0.00	. 16	2.72	5.44	7.25	4.15	1.37	.81	0.00	0.00	21.89:
1974	0.00	0.00	0.00	. 48	2.86	7.34	7.81	4.30	2.69	0.00	0.00	0.00	25.47:
1975	0.00	0.00	0.00	0.00	0.00	5.33	7.57	3.94	2.52	0.00	0.00	0.00	19.36:
1976	0.00	0.00	0.00	. 20	2.72	5.46	7.58	1.84	1.58	. 40	0.00	0.00	19.77:
1977	0.00	0.00	0.00	1.17	. 08	5.84	6.28	3.36	1.34	1.00	0.00	0.00	19.07:
1978	0.00	0.00	0.00	0.00	1.17	6.01	7.32	3.74	0.00	1.00	0.00	0.00	19.25:
1979	0.00	0.00	0.00	. 70	2.14	5.93	7.41	2.80	2.52	. 24	0.00	0.00	21.75:
1980	0.00	0.00	0.00	. 26	0.00	4.05	6.84	3.58	. 39	. 68	0.00	0.00	15.80:
1981	0.00	0.00	0.00	0.00	1.47	6.03	8.33	4.77	2.17	0.00	0.00	0.00	22.76:
1982	0.00	0.00	0.00	. 07	2.25	5.59	6.42	3.98	. 65	0.00	0.00	0.00	18.97:
AVE	0.00	0.00	0.00	. 36	1.80	5.35	7.50	3.66	1.56	. 45	0.00	0.00	20.68

Table 20.--Comparison of Results, Annual Crop Irrigation Requirement for the Weighted Crop Distribution

Item	XCIR Results	SCS Irrigation Guide	Bulletin $516\frac{1}{}$
Consumptive irrigation requirement (inches per acre)	20.68	20.29	21.42
Percent of XCIR results (percent)		98.1	103.6

<sup>1/</sup> Assumes that 8 out of every 10 years the crop irrigation requirement will be met.

The historic 20-year average (1963 through 1982) annual project farm delivery for the A&B Irrigation District was 3.06 acre-feet per acre (36.72 inches). Assuming an average annual consumptive irrigation requirement of 1.72 acre-feet per acre (20.68 inches), the average annual onfarm irrigation efficiency would be 56 percent.

# Farm Irrigation Efficiency

Initially, the majority of the project was gravity irrigated, but with the increased efficiency associated with sprinkler irrigation, approximately 30 percent of the project land has been converted to sprinkler irrigation (1983 crop census). Sprinkler irrigation properly adjusted to the soil infiltration rate is generally more efficient than gravity methods. The topography of the Minidoka North Side Extension is well suited for sprinkler irrigation, having average slopes of 2 to 4 percent. It was assumed that all newly irrigated lands in this project would be sprinkler irrigated and have an irrigation application efficiency of 70 percent.

## Redivertible Return Flows

The higher efficiency of sprinkler irrigation systems makes reuse of onfarm runoff uneconomical. The irrigators in the district exhibit good farming practices, and it was assumed the surface return flows from the new land would be insignificant and uneconomical to reuse. However, the aggregation of multiple farm runoff from the existing system may provide a portion of the irrigation demands to lands near the major drainageways within the project. The accumulated irrigation return flow and its reuse was discussed in the water supply section of this appendix.

#### Conveyance System Losses

Water losses due to seepage and operational spills are termed conveyance losses. Monthly seepage losses were computed as a percentage of the annual diversion requirements using the district's historical records as the data base (period selected was 1963 through 1982). With project conditions, the Extension land in Unit A of the district will be served by enlarging existing canals (earth-lined), whereas new concrete-lined canals will be constructed to serve the new lands in Unit B. Therefore, diversion requirements were

computed for two conditions: (1) those lands served by the enlarged existing system (Unit A) and (2) those lands served by new concrete lined canals (Unit B).

The seepage computations were based on the channel seepage equation (assumes free drainage conditions) presented in the "Drainage Manual" published by the Bureau of Reclamation in 1978. To account for the additional seepage losses attributable to the new lands in Unit A, a typical cross section was selected for computations. The seepage loss was estimated for the existing section based on the district's records from 1963 through 1982, and then a new seepage loss was estimated for the enlarged canal section. The percent change in seepage losses associated with enlarging the selected canal section (required to meet the demands of the new lands in Unit A) was assumed to represent all enlarged canal sections and were attributed to the newly irrigated lands.

The A&B Irrigation District's records were examined from 1963 through 1982 for Unit A, and the average annual seepage loss for this 20-year period was 19 percent of the average annual diversion requirement. The annual seepage loss associated with the new lands served in Unit A (enlarged canal section) was estimated at 15 percent of the annual diversion requirement for the new lands.

Operational spills result from less than 100 percent efficient operation of the irrigation system. Unavoidable losses through waterways occur due to the improper timing of releases and diversions. After reviewing the historic records for the A&B Irrigation District, it was assumed operational losses for the new lands served in the Unit A would be 6 percent of the annual diversion requirements for the new lands.

Seepage losses and operational spills for the Extension land served in Unit B should be minimal. The new canals are concrete lined and very short in length. Although the existing canals in Unit B are short in length, the losses associated with the existing system will be slightly higher than the losses in the new system because the existing canals are not lined. The 20-year (1963-82) average annual conveyance losses (seepage plus operational) in Unit B were 8 percent of the average annual ground water pumped. Accounting for the concrete lining, it was assumed the total annual conveyance losses for the new lands would be 5 percent of the annual ground water pumped to the proposed new lands in Unit B.

## Leaching Requirement

No additional water for leaching of salts would be required for any of the Extension lands. The present salinity level in the soils is very low. Irrigation water applied from either the Snake River or ground water would be of high quality.

#### Diversion Requirements

Summaries of the diversion requirements for the new land in Units A and B are presented in tables 21 and 22, respectively.

Table 21.--Diversion Requirements, New Lands Served by Unit A (inches per acre)

	Weighted Crop Irrigation	Farm	Conveya	nce Losses	Diversion
Month	Requirement	Delivery	Seepage	Operational	Requirement
April	0.36	0.51	0.10	0.04	0.65
May	1.80	2.57	0.49	0.19	3.25
June	5.35	7.64	1.45	0.58	9.67
July	7.50	10.71	2.03	0.81	13.55
August	3.66	5.23	0.99	0.40	6.62
September	1.56	2.23	0.42	0.17	2.82
October	0.45	0.64	0.12	0.05	0.81
Total	20.68	29.53	5.60	2.24	37.37

Table 22.--Diversion Requirements, New Lands Served by Unit B (inches per acre)

Month	Weighted Crop Irrigation Requirement	Farm Delivery	Conveyance Losses—	Diversion Requirements
April	0.36	0.51	0.03	0.54
May	1.80	2.57	0.14	2.71
June	5.35	7.64	0.40	8.04
July	7.50	10.71	0.56	11.27
August	3.66	5.23	0.28	5.51
September	1.56	2.23	0.12	2.35
October	0.45	0.64	0.03	0.67
Total	20.68	29.53	1.56	31.09

<sup>1/</sup> Represents operational spills plus seepage losses

The plan would serve the Extension lands from three basic sources:

(1) Snake River pumping and the use of existing North Side Pumping Division

(A&B Irrigation District) facilities, (2) the use of water from existing A&B

Irrigation District drains, and (3) ground-water pumping to serve the lands in

Unit B. The total area to be irrigated from each source and the annual

diversion requirements are presented in table 23.

Table 23.--Annual Diversion Requirements by Source

Source	Extension Area	Annual Diversion Requirement
	acres	acre-feet
Snake River supplies from North Side Pumping Division	3,950	12,300
Drainwater from existing North Side Pumping Division drains Ground water <sup>2</sup>	580 5,680	$\frac{1,500^{1/2}}{14,700}$
Total	10,210	28,500

<sup>1/</sup> Drainwater would need to be augmented with ground-water pumping. For Area 12, existing project wells would be required to augment both early and late season drainwater flows. A new well would be required to augment H Main Drain flows for much of the irrigation season to serve lands in Area 11.

2/ Does not include 240 acres of square-up lands scattered throughout Unit B. The annual diversion requirement for this land is 620 acre-feet and will be provided from existing project wells in Unit B.

## Section II--Peak Delivery Requirement

The frequency of irrigation is determined by the amount of usable soil moisture available for crop consumptive use. This study assumes no net withdrawal of usable soil moisture during the design peak consumptive use period. As a result, the number of days in the design peak consumptive use period equals the frequency of irrigation at peaking as determined by the usable soil moisture. This assumption is based on consumptive use data that indicate there can be several peak periods of about the same magnitude, and these periods can essentially occur back-to-back with little opportunity to replace any net withdrawal of soil moisture.

## Design Rate of Evapotranspiration

The design evapotranspiration rate was computed by use of the Engineering & Research Center computer program PEAKWRN (utilizing the Jensen-Haise procedure). First the program estimates daily ET, and then it identifies the peak daily moving mean evapotranspiration for a selected interval of days for each year of the period of record. The design rate for the selected interval is then defined as the mean annual peak daily evapotranspiration plus one standard deviation. The required delivery of soil moisture to the plants during the interval is then equal to the product of interval days and the design rate. The selected interval for a given crop or a weighted crop distribution equals the number of days between irrigations as determined by the usable soil moisture. This requirement assumes a zero net withdrawal of soil moisture during the peak consumptive use period. It is also assumed that the water consumptively used during the peak period will be entirely replaced by irrigation and that there will be zero effective precipitation.

Since the irrigation interval corresponding to the usable soil moisture cannot be known until the design rate is known, it was necessary to compute

design rates for several intervals and plot them against usable soil moisture. Using daily temperatures and solar radiation data, daily moving team evapotranspiration values were computed for 1-, 3-, 7-, 10-, 15-, 20-, and 30-day intervals for each crop and crop distribution. Design rates of evapotranspiration for the project area were computed by adding one standard deviation to each yearly mean peak daily evapotranspiration.

## Data Input to PEAKWRN

The basic data required in the computations of peak delivery requirements include project crop distribution, soil moisture data, daily temperature data, daily solar radiation, and crop coefficients. These parameters are thoroughly discussed in "Section I--Monthly Diversion Requirement;" however, one difference exists. The PEAKWRN program requires daily climatological data, whereas the XCIR required monthly climatological data. The availability of daily climatological data differs slightly from the monthly data used in section I in that the daily data is available from 1965 through 1980 (for the April through October period), whereas the monthly data is available from 1965 through 1982. Therefore, the period of record (1965-80) used in the peak delivery requirements will be 2 years shorter than the period (1965-82) used in the monthly diversion requirements.

#### PEAKWRN Results

A summary of the peak daily ET results are shown in table 24. These results are based on a mean project elevation of 4250 feet. The mean peak occurs in July for beans, sugar beets, alfalfa, and the weighted crop distribution, whereas, the mean peak for winter wheat occurs in June. The small grains (barley and spring wheat) mean peak occurs in June or July depending on the time interval.

Table 24.--Design Rate of Peak Evapotranspiration

Crop	Peak ET	Month of Occurrence
	(inches/day)	
Spring grain	.334	June-July
Winter wheat	.303	June
Beans	.337	July
Sugar beets	.298	July
Alfalfa	.334	July
Weighted crop distribution	.304	July

The peak daily ET for several time intervals were plotted against usable soil moisture for each crop and the weighted crop distribution. The usable soil moisture for each crop and the weighted crop distribution are discussed in section I and presented in table 15. Entering the above curves with the appropriate usable soil moisture from table 15, the design peak evapotranspiration rates were computed for each crop as presented in table 24. Comparison with Other Estimates

Peak daily ET at Rupert, Idaho, is published in the SCS "Irrigation Guide for South and Southeastern Idaho" (dated 1970) by soil type and associated slopes. A portneuf silt loam with a slope of 2 percent was selected to represent the project area. The peak daily ET for each crop and the weighted crop distribution from the above publication are listed in table 25. Table 26 compares the PEAKWRN results and the farm delivery requirement assuming a 70-percent irrigation efficiency.

Table 25.--Peak Daily Evapotranspiration (Mean Annual Plus One Standard Deviation), 1965-80

TEMPERATURE & SOLAR LOCATION MINIDOKA								
LUCATION MINIDOKA	NURTH :	SIDE EXIE	NOTUN	ELEVAII	UN 42:	O. F1		
			LENGTH	OF PERT	100 IN D	148		
CROP	1	3	5	7	10	15	20	30
POTENTIAL ET								
PEAK ET (INCHES PER DAY)	. 394	. 382	. 367	. 359	. 35 1	. 344	. 339	. 33
PEAK ET X DAYS (INCHES)	. 39	1.15	1.83	2.52	3.51	5.16	6.77	9.93
SMALL GRAINS								
PEAK ET (INCHES PER DAY)	. 392	. 38 1	. 365	. 356	. 344	. 332	. 325	. 31
PEAK ET X DAYS (INCHES)	. 39	1.14	1.82	2.49	3.44	4.98	6.49	9.30
WINTER WHEAT								
PEAK ET (INCHES PER DAY)	. 367	. 348	. 334	. 327	.318	. 305	. 294	. 27
PEAK ET X DAYS (INCHES)	. 37	1.04	1.67	2.29	3.18	4.57	5.87	8.28
BEANS								
PEAK ET (INCHES PER DAY)	. 358	. 346	. 342	. 337	. 331	. 323	. 315	. 29
PEAK ET X DAYS (INCHES)	. 36	1.04	1.71	2.36	3.31	4.85	6.31	8.90
SUGAR BEETS								
PEAK ET (INCHES PER DAY)	. 344	. 335	. 325	. 318	.311	. 304	. 298	. 29
PEAK ET X DAYS (INCHES)	. 34	1.00	1.62	2.23	3.11	4.56	5.96	8.72
ALFALFA								
PEAK ET (INCHES PER DAY)	. 386	. 374	. 364	. 356	. 348	. 338	. 332	. 32
PEAK ET X DAYS (INCHES)	. 39	1.12	1.82	2.49	3.48	5.08	6.64	9.64
MINIDOKA N.S. EXT.								
WT 31 8 8 26 27 0 0 0								
PEAK ET (INCHES PER DAY)	. 35 1	. 340	. 328	. 321	.311	. 304	. 295	. 28
PEAK ET X DAYS (INCHES)	. 35	1.02	1.64	2.25	3.11	4.56	5.90	8.52

Table 26.--Comparison of Peak Evapotranspiration and Net Farm Deliveries

	Peak Evapotran	spiration
Crop	SCS Publication	PEAKWRN
	(inches/day)	(inches/day)
Grains Beans Sugar beets Alfalfa	0.23 0.30 0.28 0.29	$0.32\frac{1}{0.34}$ 0.29 0.33
Weighted crop distribution	0.265	0.304
Farm delivery for the weighted crop distribution	0.379	0.434

 $<sup>\</sup>underline{1}/$  Average of winter wheat and spring grains

The PEAKWRN results are consistently higher for beans, alfalfa, and grains. The pertinent data associated with peak daily consumptive use computations was reviewed, and the PEAKWRN results appeared to be reasonable and within acceptable limits.

Peak farm delivery data appears in the publication "Sprinkler Irrigation in the Pacific Northwest, A Troubleshooter's Guide." In this publication a range was developed for peak farm delivery in Idaho assuming sprinkler irrigation with an operation of 24 hours per day 7 days per week with an irrigation efficiency of 70 percent. The range developed for the project area was 0.34 to 0.45 inches per day (peak ET of 0.24 to 0.31 inches per day). The peak daily ET computed in PEAKWRN and peak daily ET published in the SCS guide fall within this range.

The existing project was designed and constructed on a peak farm delivery of 0.357 inches per day, which compares with the SCS data. In a letter from the A&B Irrigation District to the Bureau of Reclamation dated May 24, 1984, the district views the development of the Extension lands as a completion of the original project and recommends a peak farm delivery no greater than 0.357 inch per day. Assuming 70-percent irrigation efficiency, this would be a peak ET of 0.25 inches per day. The newly irrigated lands will be an extension of the project; therefore, it was assumed the irrigators would continue to diversify their crops. The peak rate of 0.357 inches per day is recommended for use to develop peak delivery requirements. This rate represents the capacity required at the turnouts and is not inclusive of conveyance losses in the laterals and the canals.

#### CHAPTER 6--WATER UTILIZATION

Surface-water utilization for the Minidoka North Side Pumping Division Extension project was simulated by using the upper Snake River digital model based on the period 1928 through 1978. The Snake River model has been used in previous planning studies and subsequently was used in this study. There were no significant revisions to the model during this study; however, the data base used in the previous study was expanded to include an additional 10 years.

The model allows one to measure the impacts of any proposal on the upper Snake River system. The digital model operates the Snake River system of reservoirs and, whenever possible, provides a water supply to the users using natural flows, storage, and the rental pool. These three sources are utilized by a project or canal company in a specific order similar to the existing practices of the users. The reservoir contents and riverflow resulting under the proposed project conditions for the 51-year period examined can be seen on table 27 (units are in 100 acre-feet) at the end of this chapter.

Certain assumptions have been made which affect the values produced in the operation study. The most significant assumption involves the amount of storage space arbitrarily assigned to the Salmon Falls project which affects the yield from Ririe Reservoir. Since no final assignments have been made of Ririe storage at the time of this appendix, the ownership assignments will likely shift as the contracts are completed for the unassigned storage space.

The majority of the canals in the upper Snake River system do not have diversion records spanning the 51-year period examined (one of which is the A&B Irrigation District). To measure any impacts (such as the proposed project) on the Snake River system for an extended period, monthly diversion demands were estimated for the canals with limited records, and these estimates were based on selected canals (key canals) in the upper Snake River system which have long-term diversion records and have maintained the same irrigated area throughout the 51-year period. The monthly diversion demands computed for the existing and the new Unit A lands were based on the Twin Falls South Side Canal. These estimated demands differ slightly from historic records; however, operation model runs for the existing system indicate the total demand on the Snake River system is representative of actual practices. Further, natural flow and storage diversions were computed for the existing

Unit A lands using the model, and as illustrated in table 28, these results compare favorably with the historic operations for the 16-year period examined (the historic records and the model results overlap from 1963 through 1978).

Table 28.--Comparison of Results for Existing Unit A Lands (1963 through 1978)

	Natural Flow Diversions	Storage Diversions
Historical Model	acre-feet 21,740	34,890
Mode I—	22,430	34,200

<sup>1/</sup> Assumes no shortages. The district's records indicate they did not experience a shortage during this period.

To effectively evaluate the impacts of the proposed new Unit A lands on the district's surface-water supply and considering the complexity of the model, the monthly irrigation demands of the new Unit A lands and the existing Unit A lands were combined and not treated as separate diversions. The irrigation diversions of the new lands represent about 18.2 percent of the combined irrigation demands. A summary of the model results with the combined irrigation demands are presented in table 29 (at end of this chapter).

The main intent of using the digital model is to ascertain the impacts on the district's surface-water rights and, more importantly, to assure an adequate water supply during dry periods. Under Idaho law, the district can only divert natural flow from the Snake River to the existing Unit A lands, however, due to constraints in the digital model, natural flow water was diverted to the new Unit A lands when it was available. The results presented in table 29 were examined, and this inconsistency had negligible effects during dry periods but provides a significant part of the diversion requirements of the new Unit A lands in wet years. Even though this

inconsistency has minimal effects during dry periods, the water supply was recomputed on an annual basis, assuming no natural flow diversions to the new lands. These results are presented in table 30, and selected years are summarized in table 31.

As previously discussed, the main factor which enables the district to provide a dependable water supply to the new land is the refill capabilities of American Falls. Under project conditions, the district received their full American Falls allocation in 49 out of the 51 years examined. In the 2 years (1934, 1960) the district did not receive a full allocation, they received 90 percent of the full allocation in 1 year and 98 percent in the other.

In 47 years of the 51-year period examined, part or all of the storage water diverted to the new Unit A lands came from American Falls Reservoir. The average American Falls storage diverted to the new lands over the 51-year period was 9,666 acre-feet or about 80 percent of the total storage water diverted to these lands. The model results indicate a dependable water supply can be provided to the proposed new Unit A lands under the district existing storage water rights without producing any significant impacts on the existing Unit A lands. The shortages which occur are voluntary in nature and are well within the accepted criteria adopted by the Bureau of Reclamation. A full discussion of the voluntary shortages is discussed later in this chapter.

Table 30.--Annual Summary, North Side Pumping Division Operation Study

Makan			Water		American	Palisades
Water Year	Irrigation Demand	Shortage <sup>2</sup> /	Natura) Flow	Storage Water	Falls Storage Used	Storage Used_
1601	Demand-		acre	-feet	Scorage used	0360-
1928	68,660	0	34,807	33,853	33,853	0
1929	67,060	ő	26,269	40,791	40,791	ő
1930	63,380	Ō	14,186	49,194	46,826	2,368
1931	62,490	0	10,024	52,466	46,826	5,640
1932	63,340	0	20,269	43,071	43,071	0
1933	69,240	0	21,970	47,270	46,826	444
1934	57,736	-14,511	3,690	39,535	39,535	0
1935	63,008	-21,336	18,900	22,772	22,772	0
1936	67,660	0	21,670	45,990	45,990	0
1937	65,700	0	16,955	48,745	46,826	1,919
1938	65,190	0	26,098	39,092	39,092	0
1939	68,910	0	12,000	56,910	46,826	10,084
1940	66,200	0	10,559	55,641	46,826	8,815
1941	64,090	0	5,638	58,452	46,826	11,626
1942	64,290	0	11,319	52,971	46,826	6,145
1943 1944	66,220 62,110	0	32,842 14,729	33,378 47,381	33,378 46,826	0 555
1945	65,070	0	25,238	39,832	39,832	0
1946	65,910	Ö	27,488	38,422	38,422	0
1947	66,370	.0	27,367	39,003	39,003	0
1948	67,640	Ö	31,270	36,370	36,370	ő
1949	67,410	ő	21,410	46,000	46,000	ŏ
1950	68,350	Ö	30,612	37,738	37,738	Ô
1951	68,170	ō	26,596	41,574	41,574	Ö
1952	72,100	Ō	28,156	43,944	43,944	0
1953	70,240	0	20,724	49,516	46,826	2,690
1954	69,780	0	27,121	42,659	42,659	0
1955	68,360	0	22,249	46,111	46,111	0
1956	68,800	0	23,094	45,706	45,706	0
1957	67,710	0	24,926	42,784	42,784	0
1958	68,010	0	19,044	48,966	46,826	2,140
1959	66,350	0	28,744	37,606	37,606	0
1960	68,950	0	2,190	66,760	46,826	19,934
1961	65,410	0	7,630	57,780	45,687	12,093
1962	70,360	0	24,844	45,516	45,516	0
1963	69,260	0 0	23,486 28,956	45,774	45,774 42,354	0
1964 1965	71,310 71,610	0	28,553	42,354 43,057	43,057	0
1966	72,140	0	4,530	67,610	46,826	20,784
1967	73,320	ŏ	25,988	47,332	46,826	506
1968	70,820	ő	12,820	58,000	46,826	11,174
1969	72,240	ŏ	9,598	62,642	46,826	15,816
1970	68,820	Ŏ	27,140	41,680	41,680	0
1971	70,920	Ō	28,826	42,094	42,094	0
1972	70,470	0	28,533	41,937	41,937	0
1973	68,450	0	11,782	56,668	46,826	9,842
1974	69,540	0	27,750	41,790	41,790	. 0
1975	66,090	0	28,952	37,138	37,138	0
1976	69,760	0	29,363	40,397	40,397	0
1977	60,346	-15,636	6,642	38,068	38,068	0
1978	63,080	0	35,984	27,096	27,096	0
 Average	67,421	-1,010	21,167	45,244	42,4481/	2,796
Average without shortages <u>2</u> /	67,421	0	21,167	46,253	43,089	3,164
shur cayes—	07,421	J	61,10/	70,233	73,003	3,104

 <sup>1/</sup> This value represents the combined irrigation demands of the existing Unit A land (81.8 percent) and the proposed new Unit A lands (18.2 percent).
 2/ Irrigation shortages are voluntary in nature. Model results indicate there was sufficient storage in the 3 years to meet these shortages.
 3/ Natural flow is only diverted to the existing Unit A lands

Table 31.--Summary of Results in Acre-feet, 1928 through 1978

		Existin	Extension Unit A Lands								
Event 1/	Irrigation Regulrement	Shortage <sup>2</sup> /	Natural Flow Diversion	Storage American Falls	Palisades 3/	Irrigation Requirement	Shortage1/	Natural Flow Diversion	American	Diversions Palisages 3	
EASIG-	Requirement	Jiidi cage	V1461310H	14113 /	781134443-	Requirement	Shor cage	0116131011		reiliages-	
det year (1943) Ory year (1934)	54,141	υ	32,842	21,299	0	12,079	0	0	12,079	U	
With voluntary shortages	47,205	-11.864	3,690	31.651	0	10,531	-2.647	0	7,884	U	
Without voluntary shortages	47,205	0	3,690	42.131	1.384	10.531	0	Ó	0	10,531	
Typical year (1958) Nverage 1928-78	55,605	Ō	19,044	36,561	0	12,405	Q	Ö	10,265	2,140	
With voluntary shortages	55,123	-826	21,167	32,782	348	12,298	-184	0	9,666	2,448	
Without voluntary shortages	55,123	0	21.167	33,956	348	12.298	0	Ö	9,481	2,816	

## Rental Pool

One main objective of the digital model was to maximize water utilization on the upper Snake River system. Projects and canal companies with the ability to contribute water to the rental pool were examined, and an arbitrary annual contribution was established for each entity. This contribution represents the maximum amount of storage water which can be contributed to the rental pool without producing significant impacts on the existing water supply. For the A&B Irrigation District, it was assumed all Palisades storage water in excess of 32,100 acre-feet (district's Palisades storage right is 90.800 acre-feet) would be made available to the rent pool. This rental pool limit value has remained the same in this study as it has throughout all previous planning project studies.

In the digital model, the entities storage water is reduced to the established rental pool limit during the month in which storage water is allocated to all users. Then, at the termination of each irrigation season, the unused portion of the rental pool is credited back to each contributing entity.

Presently, the A&B Irrigation District contributes 50,000 acre-feet of Palisades storage to the rent pool annually. An examination of the model results indicates the district could continue to contribute to the rent pool

<sup>1/</sup> Years selected were based on the historic date from 1928 through 1978 for the Snake River at Heise
2/ Shortages are voluntary in nature and the digital model results indicate the district has siffucient storage space to adequately meet all of
the irrigation requirements for the 51-year period examined
3/ Prior to 1978, a conveyance loss was charged to Palisades storage when it was utilized. This has been discontinued and these values reflect
current this change.

and provide a dependable water supply to the proposed new Unit A lands. However, it may be prudent for the district to modify their contribution during dry periods.

## Shortages

There are three dry periods during the 51-year period examined, occurring in 1934, 1935, and 1977. In these years, voluntary water shortages were assigned to each project or canal company and were implemented to maximize water utilization and carryover storage. The shortages are input as a percentage of the monthly irrigation demands, and it was assumed the same percentage would apply to both the new and the existing Unit A lands. It should be emphasized that these shortages are only voluntary in nature, and the digital model results (table 29) indicated the district had sufficient storage to adequately meet all of the irrigation demands during the 3 years where voluntary shortages were implemented. Further, the district contributed storage water to the rental pool in all 3 years, and based on the model results the district should not experience water shortages under the Recommended Plan. During dry periods the district could modify or eliminate their contribution to the rental pool which would assure a full water supply to the existing system and the proposed project.

# Effects on the Upper Snake River System

Providing full irrigation service to lands within the A&B Irrigation District would have a negligible impact on upper Snake River system surface-water supplies. The Recommended Plan includes the use of Snake River water to provide full irrigation service to 3,950 acres  $\frac{1}{}$  of land located in

<sup>1/</sup> Includes about 3,140 acres of new irrigated land and 810 acres of existing irrigated land requiring a replacement water supply.

Unit A of the A&B Irrigation District. Ground water would be pumped from the Snake Plain aquifer to provide full irrigation service to 5,680 acres of land in Unit B. As previously discussed, surface return flow under the Recommended Plan would be insignificant; however, irrigation losses due to deep percolation will ultimately end up in the Snake Plain aquifer.

Pumping from the Snake Plain aquifer does not approach recharge. Present pumping is estimated at 1.4 million acre-feet per year, and recharge is estimated at about 8 million acre-feet per year. The estimated net average annual ground-water requirement for the new Unit B lands is 13,520 acre-feet, about 1 percent of the present Snake Plain aquifer pumping. Under the Recommended Plan, the aquifer should accommodate the proposed ground-water pumping without showing any significant declines. Further, the effects of the additional pumping will not be identifiable at Thousand Springs.

The estimated annual surface-water requirement for the new Unit A land is 12,300 acre-feet and represents less than 0.5 percent of the 2.9 million acre-feet of combined active storage in these two reservoirs. Such a small percentage of the total storage could be released without measurably affecting water surface elevations at the two reservoirs.

Assuming an average annual diversion requirement of 12,300 acre-feet, the maximum increase in Snake River flows below American Falls Reservoir would be about 60 ft<sup>3</sup>/s during the peak irrigation period. The average peak irrigation flow below American Falls is currently about 12,200 ft<sup>3</sup>/s; with the Extension project, this flow would increase by about 0.5 percent. This slight increase in flow is not significant. Comparing an average annual diversion requirement of 12,300 acre-feet to an average annual diversion of 8.2 million acre-feet (period of record 1928 through 1981) for the upper Snake River system, there would be less than a 0.2-percent change in total diversion requirements. Dry

and wet periods were also examined for these two cases, and the relative impacts on the upper Snake River system remained negligible.

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# Table 27.--Column Description, Snake River Flow and Reservoir Contents

Column	Description
WYR	Water year of operation study
MO	Month of operation study
FCSPSN	Flood control space required on the main stem of the Snake
7 001 011	River (hundreds of acre-feet)
FRCSTH	Heise runoff forecast from the beginning of the given month
	to July 31 (hundreds of acre-feet)
SYSCON SNAKE	The summation of the end-of-month contents of Jackson Lake, Palisades, and American Falls Reservoirs (hundreds of
	acre-feet)
SYSCON H. FORK	The summation of the end-of-month contents of Island Park
	Reservoir, Grassy Lake, and Henrys Lake (hundreds of
	acre-feet)
RENT POOL	Amount of water in the system which is available to be
	rented out to canal companies in need of supplemental water
	(acre-feet)
XCUT CANAL	Water diverted by the Crosscut Canal into the Teton River
	(acre-feet)
GAIN USED BY	Reach gains and return flows diverted to the Minidoka and
MINDKA BURLEY	Burley Irrigation Districts. These gains and return flow
TOTAL VOLET	occur in reaches 108 and 109 of the Snake River (acre-feet)
TOTAL XCUT	Total water diverted into the Crosscut Canal from the
	Henrys Fork of the Snake River and the Falls River
H.FORK SMSHRT	<pre>(acre-feet) The summation of irrigation shortage incurred by the canals</pre>
H.FUKK SMSHKI	in the Henrys Fork of the Snake River (acre-feet)
JACKSON LAKE	(hundreds of acre-feet)
INFLOW	Inflow to Jackson Lake
EOM	End-of-month content of Jackson Lake
OUTFLO	Outflow from Jackson Lake
PALISADES	(hundreds of acre-feet)
INFLOW	Inflow to Palisades
EOM	End-of-month content of Palisades
OUTFLO	Outflow from Palisades
HEISE FLOW	Snake River flow near Heise (hundreds of acre-feet)
HENRYS LAKE	(hundreds of acre-feet)
EOM	End-of-month content of Henrys Lake
OUTFLO	Outflow from Henrys Lake
ISLAND PARK	(hundreds of acre-feet)
INFLOW	Inflow to Island Park
SEEPAGE	Island Park seepage loss End-of-month content of Island Park
EOM	Outflow from Island Park
OUTFLO H.FORK ASHTON	Flow in the Henrys Fork of the Snake River near Ashton
n.rukk ASHIUN	(hundreds of acre-feet)
GRASSY LAKE	(hundreds of acre-feet)
EOM	End-of-month content of Grassy Lake
OUTFLO	Outflow from Grassy Lake
J J	· · · · · · · · · · · · · · · · · · ·

Table 27.--Continued

Column	Description
EL DIV CUECTED	Fall Diver flow page Chapter (bundants of page fact)
FL RIV CHESTER	Fall River flow near Chester (hundreds of acre-feet)
H.FORK STANTY	Flow in the Henrys Fork at the Snake River near St. Anthony
TT DIV CTANTY	(hundreds of acre-feet)
TT RIV STANTY	Teton River flow near St. Anthony (hundreds of acre-feet)
H.FORK REXBRG	Flow in the Henrys Fork of the Snake River near Rexburg
CUELLY ELOU	(hundreds of acre-feet)
SHELLY FLOW	Snake River flow near Shelly (hundred of acre-feet)
BLFT FLOW	Flow in the Snake River near Blackfoot (hundred of
	acre-feet)
AMERICAN FALLS	(hundreds of acre-feet)
INFLOW	Inflow to American Falls
EOM	End-of-month content of American Falls
OUTFLO	Outflow from American Falls
LAKE WALCOTT	(hundreds of acre-feet)
INFLOW	Inflow to Lake Walcott
EOM	End-of-month content of Lake Walcott
OUTFLO	Outflow from Lake Walcott
MILNER FLOW	Snake River flow past Milner Dam (hundreds of acre-feet)
RIRIE	(hundreds of acre-feet)
INFLOW	Inflow to Ririe
EOM	End-of-month content of Ririe
OUTFLO	Outflow from Ririe
DIVRTD	Option not used Ririe water diverted to Progressive Irrigation District's
DIVRTD	<u> </u>
DUMD	old lands (acre-feet) Option not used
PUMP	Flow in reach 148 of the Snake River (hundreds of
FLOW148	acre-feet)
OPCODES	Values establishing operating limits for the system. The
UPCODES	codes set operation limits for the eight reservoirs
	presented, both in content and downstream reaches.
	presences, both in content and dominour cam readines.

RIGHTS																																							
S 1939	H. FORK Smshrt	o c	ö	o.	o	o.	o.	Ö	-531.	-92.	Ö	0	-623.	o	o O	Ö	o.	0	o O	Ö	0	0	Ö	0	ö	o.	°.	Ö	Ö	ö	ö	o.	o.	o O	o O	o O	o O	ö	o
INCLUDES	TOTAL XCUT	o c	; o	0	o O	0	o O	0	o O	ö	8580.	ó	8580.	ö	Ö	o.	o O	0	o	o	ö	o	7207.	12519.	o.	19725.	o O	ö	o	o O	o O	o.	Ö	o O	947.	9267.	4197.	1776.	16187.
FALLS,	USED BY (A BURLEY	o c	ó	0	ö	Ö	Ö	Ö	ö	5600.	3360.	o.	8960.	o O	ö	Ö	ö	Ö	ö	ö	ó	2996.	Ö	5075.	19775.	27846.	o O	Ö	o o	ö	ö	o O	Ö	Ö	Ö	105.	2940.	8085.	11130.
SALMON FALLS,	GAIN US MINDKA	o c	ó	Ö	0	ö	o O	Ö	o O	o O	6240.	0	6240.	o.	Ö	Ö	o.	o O	Ö	o.	ö	ö	o.	9425.	36725.	46150.	0	o.	o O	ö	o O	o.	ö	ó	o	195.	5460.	15015.	20670.
1928-78 W/O	XCUT	o c	Ö	Ö	Ö	o O	o.	Ö	ö	Ö	Ö	ö	ò	0	ö	0	o O	o.	o.	o.	ö	o O	o O	Ö	ö	ó	·	ö	ö	o.	o.	ö	o O	o.	ö	o O	o O	ö	o O
1928-	RENT	o c	i o	0	ó	Ö	0	o O	60158.	.316559.	157368.	126322.	60406.	ö	Ö	Ö	ö	ö	ö	ö	22153.	335306.	270928.	105338.	82843.	16569.	ö	Ö	ö	ö	ö	0	0	313911.	.313517.	.307593.	. 306918.	. 294629.	****
AODEL	SYSCON H. FORK	1774.	2063.	2133.	2174.	2345.	2345.	2345.		2167.3		1784.	25406.660406	1806.	1815.	1824.	2040.	2054.	2234.	2290.		•	•	1885.	1704.	24420.816569	1726.	1868.	1889.	2125.	2144.	2336.	•		2283.3	1758.3	1698.3	1768.2	24184.*****
RIVER DIGITAL MODEL	SYSCON	22541.	30021.	32788.	35825.	37211.	37660.	39128.	39929.	34467.	25981.	21798.	383693.	22174.	25072.	27795.	31285.	34729.	37571.	38514.	39804.	40351.	31779.	22257.	19589.	370921.	20338.	22827.	27067.	30584.	34679.	36215.	37923.	38276.	35603.	27278.	22337.	18746.	15680.351872.
NIVER DI	FCSPSN FRCSTH	o c	Ö	4490.	4372.	4244.	4 104.	3794.	1571.	648.	ö	o	23223.383693	Ö	o.	o.	3393.	3097	. 2866.	2638.	2401.	1294.	443.	o.	o	16132.370921	Ö	ö	Ö	3380.	3098.	2885.	2655.	2195.	1085.	382.	ö	ö	15680.3
SNAKE F	FCSPSN	o c	Ö	ó	Ö	0	600.	1180.	340.	40.	o O	ö	2160.	o	o.	Ö	ò	ö	o.	o.	90.	o.	o O	ö	Ö	90.	0.	ö	ó	ö	ö	o	Ö	o.	o O	ö	ö	ö	o
UPPER	ÃÔ.	-0	, m					<b>&amp;</b>			_	12				က		ស						_	12		-					ဖ				•	•	12	
UP	WYR MO	28	28	28	28	28	28	28	28	28	28	<b>58</b>		29	59	29	29	29	53	59	59	59	29	59	29		30	ဓ္ဓ	30	ဓ္ဓ	30	30	30	30	30	30	30	30	

	SY LAKE OUTFLO	დ. 4		4	₹.	4	w.	36.	22.	4	6	29.	138.	Ö	Ö	0	ö	0	ö	ö	4	27.	<b>⊙</b>	2	27.	83.	o.	Ö	Ö	o O	Ö	Ö	Ö	•	5	M	₹.	26.	56.
	GRASSY EOM OL	152.	152.	152.	152.	152.	152.	152.	152.	152.	152.	130.	1802.	132.	135.	139.	142.	145.	148.	150.	152.	152.	152.	152.	130.	1726.	132.	134.	137.	139.	141.	143.	144.	152.	152.	152.	152.	130.	1705.
1 - K	H. FORK Ashton	871.	733.	682.	668.	636.	876.	1655.	1030.	984.	944.	941.	10730.	817.	785.	739.	519.	678.	566.	754.	1410.	1084.	918.	943.	872.	10086.	713.	547.	692.	446.	597.	514.	1129.	977.	686.	1121.	740.	528.	8690.
	OUTFLO A	358.	246.	246.	230.	153.	354.	711.	446.	461.	461.	446.	4303.	311.	282.	246.	31.	222.	61.	195.	493.	446.	461.	461.	446.	3656.	278.	112.	246.	31.	222.	67.	412.	461.	281.	723.	369.	163.	3365.
	PARK	791.	1068.	1138.	1179.	1350.	1350.	1350.	1319.	1172.	1044.	935.	13690.	926.	926.	925.	1131.	1135.	1304.	1350.	1350.	1334.	1122.	967.	853.	13322.	844.	926.	946.	1150.	1161.	1350.	1350.	1256.	1288.	787.	758.	898.	12743.
RIGHTS	I SLAND SEEPAG	ö	Ö	o.	ö	ö	ö	Ö	ö	ö	ô	ö	ö	ö	ö	Ö	ö	ö	o O	Ö	o.	ö	o O	Ö	ö	ó	ö	Ö	ö	Ö	ö	o O	o O	o O	ö	Ö	ö	o ·	ó
1939	INFLOW S	358.	320.	316.	271.	324.	366.	733.	447.	356.	370.	357.	4610.	311.	282.	245.	237.	226.	231.	252.	515.	462.	291.	341.	351.	3743.	278.	224.	236.	235.	233.	256.	424.	389.	343.	260.	371.	320.	3569.
INCLUDES	LAKE	95	25.	25.	24.	25.	27.	96	50.	<del>.</del> 5	61.	<b>.</b> <b>0</b>	420.	o.	o.	ö	o O	ö	Ö	o.	Ö	45.	₽.	61.	.09	175.	ö	ö	ö	o.	22.	30.	29.	33.	39.	ö	61.	.09	274.
FALLS,	HENRYS EOM O	830.	843.	843.	843.	843.	843.	843.	843.	843.	778.	718.	9914.	748.	754.	760.	767.	774.	782.	790.	822.	843.	843.	.997	722.	9372.	751.	779.	807.	836.	843.	843.	843.	843.	843.	819.	788.	740.	9736.
SALMON	HE I SE FLOW	2180.	3033.	2539.	775.	858.	3282.	17435.	10697.	7591.	6451.	4233.	62536.	2127.	2327.	2129.	1121.	746.	826.	1467.	5987.	10464.	7958.	6931.	3190.	45273.	2096.	1536.	1128.	1096.	721.	858.	2710.	5858.	8860.	7089.	4049.	4119.	40120.
.78 W/0	S OUTFLO	1845.	2749.	2269.	575.	615.	3098.	16472.	10086.	7168.	6107.	3988.	58 103.	1845.	2097.	1921.	922.	555.	635.	1350.	5591.	9890.	7646.	6646.	2935.	42034.	1845.	1339.	922.	922.	555.	.099	2478.	5454.	8362.	6801.	3704.	3880.	36922.
1928-78	ALISADES EOM OL	13382.	12429.	11953.	12748.	13790.	13640.	13174.	13745.	14000.	12411.	11911.	156089.	12266.	12013.	11760.	12166.	12891.	13548.	14000	13937.	14000.	12174.	9555.	10216.	148526.	10324.	10562.	11287.	11545.	12086.	12813.	14000.	14000.	14000	12542.	13234.	12759.	49151.
MODEL	P/ INFLOW	2684.	2273.	1793.	1370.	1657.	2980.	16053.	10723.	7500.	4588.	3530.	57804.	2226.	1844.	1668.	1329.	1280.	1292.	1834	5576.	10020.	5894.	4087.	3632.	40681.	1975.	1577.	1648.	1180.	1096.	1387.	3696.	5502.	8430.	4		4	39821.1
DIGITAL N		440.	494.	31.	29.	31.	29.	3035.	2702.	1845	1845.	1488.	12578.	315.	238.	336.	31.	78.	31.	30		4	1845.	4	œ	9155.	225.	144.	275.	3 <del>1</del> .	œ	31.	30.	-	8	1845.	1845.	1488.	7915.
RIVER DI	JACKSON LAKE LOW EOM OUTFLO	4965.	4965.	5335.	5570.	5864.	6301.	8054	8284.	8 156.	6896.	2	75089.	5733.	5733.	5733.	5997.	6242.	6484.	6802.	8267.	8470.	7782.	6410.	23	78885.	5232.	5232.	5232.	5442.	5678.	5873.	6515.	8245.	8470.	7620.	6408.	5229.	75176.
SN	JACH	440.	494.	400	264.	325.	200	4850.	3033.	1829.	692.	389.	13826.	350.	238.	336.	295.	273.	272.	383	1557.	3245.	1268.	579.	373.	9169.	260.	144	275.	241.	264	226.	705.	1920.	2174.	1106.	739.	372.	8426.
UPPER	WYR MO	28 1	<b>6</b>	80		<b>8</b>	<b>80</b>	<b>.</b>		- ·	28 11	- 0		29 1	ים פוס	ה כ	י ת		י עב	<b>n</b> (	<b>.</b>	GD (		י עס	<del>-</del>		0	0	0	0	0	0	0	0	0	0	30 11	-	

	INCLUDES 1939 RIGHTS
	1928-78 W/O SALMON FALLS, IN
	RIVER DIGITAL MODEL
•	UPPER SNAKE

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MILNER	728.	1327.	2201	1486.	3562.	2956.	12598.	2579.	200	200.	50.	29481.	770.	1415.	1632.	566.	112.	1547.	3289.	o.	ö	200.	200.	50.	9781.	147.	1224.	758.	288.	388.	2296.	o	ö	ö	200.	200.	50.	5551.
OUTFLO	1976.	2012.	1020.	1325.	3490.	4159.	17259.	7622.	5705	5546	3462.	56166.	2010.	1979.	1542.	400.	106.	1473.	3989.	4627.	5061.	5780.	5542.	3164.	35673.	1238.	1815.	702.	201.	225.	2402.	3036.	3724.	4693.	5925.	4854.	3031.	31846.
E WALCOTT	669	966	. 6	970.	970.	970.	970.	970.	952	952.	669	10522.	700.	700.	700.	700.	700.	970.	970.	951.	951.	952.	952.	.669	9944.	700.	700.	700.	700.	700.	970.	950.	951.	951.	952.	952.	.669	9924.
LAKE INFLOW	2879.	2012.	2260	1325.	3490.	4639.	18877.	9105.	7407	7181.	4372.	65169.	2602.	1980.	1542.	400	106.	1743.	4196.	5891.	6581.	7456.	7140.	4044.	43680.	1800.	1816.	702.	201.	225.	2672.	4164.	4608.	6297.	7663.	6241.	3885.	40273.
LLS OUTFLO	2882.	1883.	7760	1359.	3556.	4694.	18827.	8901.	7355	7176.	4375.	64691.	2420.	1842.	1485.	30.	42.	1549.	3925.	5836.	6579.	7557.	7155.	3880.	42299.	1767.	1725.	644.	30.	27.	2560.	4140.	4613.	6366.	7670.	6303.	3951.	39795.
AMERICAN FALLS Flow eom ou	3676.	•	17 109 .	17000	17000.	17000.	17000.	17000.	11411	5774.	3254.	66309.144197.	3674.	6815.	9802.	12624.	15101.	17000.	17000.	16700.	16981.	10923.	5393.	3242.	135255.	4282.	6519.	10048.	13091.	16415.	17000.	16890.	15410.	12440.	6405.	1979.	41.	38550, 120518.
Z	2792.	6179.	10.49.	3359.	3556.	4873.	19098.	9284.	2378	2021	2060.	66309.	2922.	4982.	4472.	2851.	2519.	3448.	4 104.	5798.	7237.	2101.	2109.	1917.	44461.	2892.	3962.	4172.	3073.	3351.	3145.	4203.	3392.	3734.	2181.	2285.	2159.	38550.
BLKFT FLOW	649.	4289.	2000	1572.	1474.	3086.	17526.	7455.	527.	307.	297.	44067.	934.	3122.	2816.	1326.	993.	1541.	2115.	3598.	5515.	307.	307.	297.	22872.	888	2132.	2391.	1247.	1652.	1385.	2551.	1630.	2027.	307.	307.	297.	16815.
SHELLY FLOW	1504.	4157.	255.	1521.	1587.	3641.	18930.	8829.	2413.	1930.	1614.	53742.	1977.	3117.	3190.	1655.	1333.	1414.	2540.	5108.	7677.	2209.	1954.	1386.	33561.	1623.	1976.	2154.	1563.	1338.	1414.	3336.	3114.	4023.	2349.	1581.	1503.	25975.
H. FORK Rexbrg	1439.	1338.		1038	1051.	1178.	4121.	1935.	981.	619.	1072.	17156.	1131.	1306.	1180.	777.	875.	901.	1131.	1809.	2047.	691.	277.	907.	13033.	1001.	1009.	1128.	717.	890.	872.	1326.	729.	381.	253.	365.	331.	9008
TT RIV OUTFLO	222.	305.	30 F	289.	389.	457.	1561.	535.	313.	132.	143.	4978.	156.	240.	247.	235.	209.	369.	352.	400	513.	192.	87.	124.	3124.	158.	217.	262.	226.	254.	364.	492.	225.	141.	84.	120.	94.	2637.
TT RIV STANTY	391.	360.		290.	390.	492.	25	1052.	792.	462.	382.	7186.	340.	300.	260.	240.	210.	370.	387.	764.	1096.	615.	405.	363.	5350.	4	7	7	က	ູ	65	~	ന	O	4	4	354.	4750.
H. FORK STANTY	1218.	1156.	976	890.	851.	1008.	3039.	8	768.	m	933.	13838.	1015.	1091.	974.	677.	813.	707.	898.	1855.	1825.	603.	420.	762.	11641.	9	7	90	4	2	43	70	4	9	6	က	264.	8212.
FL RIV CHESTR	406.	0 0	n u	65	80	7	98	80	8	~	_	6441.	287.	343.	304.	305.	83	318.	320.	969	1226.	29	77.	146.	4807.	230.	220.	250.	63	217.	67	635.	580.	360.	77.	7.	56.	3062.
WYR MO	28 1	<b>2</b> 0 c	) a		80	æ	83	æ	89	8	8		29 1	6	6	თ	თ	<b>о</b>	6	თ	6	<del>т</del>	6	6		30		30							-	_	-	

.OW148	22.	54	<u>5</u>	56.	7	<del>1</del> 5.	5	59.	. 96	38.	23.	19.	426.	-		<u>1</u>	14	15	5	139.	87.	37.	22.	21.	433.	5.	43.	15.	36.	15	<u>1</u> 5	15.	15.	16.	16.	15.
PUMP FLOW148	0	o.	Ö	o.	ö	o O	o.	o	o.	o.	Ö	ö	Ó	o	ó	Ö	o.	o.	o	o	Ö	o O	o O	o.	Ö	o.	o O	o.	ö	o.	ö	o.	ö	o O	ö	ö
DIVRTD	144.	21.	ö	o.	ö	ö	o.	97.	628.	919.	751.	551.	144.	21.	o	o	Ö	ö	ö	97.	517.	946.		404.	144.	21.	Ö		o.	ö	o.	-	489.	6	343.	8
DIVRTD	Ö	0	o	o O	o O	o.	o O	0	o.	o O	o.	ö	Ö	Ö	0	0	o.	o.	o.	ö	ó	ö	ö	o.	o.	o.	o O	0	o.	o O	ö	o.	ö	Ö	ö	·
OUTFLO 0	15.	52.	15.	56.	14.	15.	15.	59.	.96	39.	23.	18.	425.	15.	29.	15.	14.	15.	15.	139.	87.	37.	22.	21.	432.	5	43.	15.	36.	15.	15.	<del>1</del> 5.	15.	15.	15.	<del>1</del> 5.
RIRIE EOM (	517.	500.	517.	500.	508.	557.	719.	900	900.	900	900.	900.	500.	511.	500.	498.	495.	539.	712.	900	900.	900.	900	900	500.	514.	500.	506.	200	529.	518.	621.	693.	711.	717.	717.
INFLOW	33.	35.	32.	39.	22.	65.	7	241.	97.	40.	24.		26.	26.	18.	13.	<u>-</u>	.09	188.	328.	88.	38.	23.	22.	33.	29.	29.	21.	30.	44.	IJ.	119.	88.	35.	22.	<del>1</del> 6.
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	28	28	28	28	28	<b>58</b>	28	28	<b>58</b>	28	58	28	29	53	58	29	59	53	59	59	59	58	53	59	30	30	30	30	9	9	9	30	30	ဓ္တ	30	30

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LAKE	o.	ö	o O	Ö	Ö	Ö	Ö	Ö	Ö	Ö	Ó	130.	o.	Ö	o	Ö	o.	Ö	o.	Ö	Ö	Ċ	o	130.	Ö	ó	Ö	Ö	Ö	o.	o.	o	0	o.	o O	130.
GRASSY (	o.	ö	ö	ö	0	Ö	ö	0	o	Ö	Ö	ö	o.	ö	Ö	Ö	ó	ö	o	Ö	Ö	Ö	Ö	Ö	o.	ó	Ö	Ö	Ö	Ö	Ö	Ö	Ö	ö	ö	•
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LAKE	Ö	o	Ö	Ö	o.	Ö	Ö	o	o	Ö	Ö	Ö	Ö	Ö	Ö	Ö	Ö	Ö	Ö	Ö	ó	Ö	ó	ó	o.	Ó	ó	Ö	o.	Ö	ó	Ö	Ö	o	o.	o.
HENRYS LA	ö	o O	0	o.	o.	o.	o.	0	0	Ö	<b>1</b> 00	100	o O	o.	o.	o.	ö	o O	ö	ö	Ö	ö	100	100	Ö	Ö	o O		0	o O	Ö	ö	0	o.	<b>1</b> 00	100.
HEN	37	28	53	53	23	53	53	53	37	37	12	12	37	58	12	12	12	12	12	9	37	37	12	12	37	58	12	12	12	53	53	53	37	37	12	12
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SLAND PARK	o O	ر م	400.	400	60.	<b>1</b> 00	o O	750.	750.	750.	750.	750.	o.	o.	400.	50.	400.	<del>1</del> 00.	o O	750.	750.	750.	750.	750.	o.	ъ.	400.	20.	400.	<u>\$</u>	ö	750.	ö	750.	ö	ö
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OPCODES JACKSON LAKE	o.	o O	ö	0	o.	o.	o.	0	o.	. 8	8	8		ö	o.	٥.	o O	o O		ö	0.84	8	8	8.	o.	o.	o o	٥.	0	٥.	ö	0.84	8	8	ė.	ė.
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1928-78 W/O SALMON FALLS, INCLUDES 1939 RIGHTS

UPPER SNAKE RIVER DIGITAL MODEL

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SALMON F	HEISE	FLOW	2170.	2514.	1118.	1093.	1175.	1455.	2174.	SOR6	5806.	4225	4185.	3847.	34847.	2636	1504	. 600	1000	1439.		1047.	735.	7152.	9038.	6705.	6172.	3712.	42317.	2521.	1256.	1116.	1126.	948.	1399.	2126.	4789.	12366.	6523.	5836.	4208.	44215.
28-78 W/O S		OUTFLO	1845.	2289.	922.	922.	1016.	1271.	2043	4806	7000.	4059	3987	3680.	32319.	2444			927	1099.	.030	8/6.	595.	6612.	8350.	6331.	5829.	3477.	38983.	2278.	1041.	922.	922.	781.	1225.	2003.	4464.	11653.	6249.	5561.	3996.	41096.
1928-7	ISADES	EOM OL	13455.	12964.	13610.	13881.	14000.	14000	14000	. 000		12295	12477	11551.	61234.	10641	1001		11021.	10966.	10881.	11109.	12744.	13594.	13745.	14000.	12441.	12340.	44098.	11944.	12624	13257.	13690.	14000.	14000.	14000.	13944.	13940.	12552.	10717.	9804.	154472.
MODEL	PAI	INFLOW	2568	1798.	1568	1194.	1135.	1271	2075	A D FA	1001	0 4		2796.	31472.1	u u			1327.	1044.	945.	1104.	2259.	7509.	8568.	6663.	4339.	3419.	40126.1	1908	1721	1556.	1355.	1091	1225.	2035.	4456.	11716.	4936.	3790.	3120.	38908.
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	AMERI	INFLOW	2717	4659.	3536	2063	, 2000 2000 2000		3000	3602.	2923.	2363.	2221.	3097.	3042.	37920.	2249.	2577	2928	2850	2627		3036.	2349.	6236.	7213.	2646.	2326.	2169.		39226.	2170	3064	26.46			2003.	3392.	3911.	5228.	7743.	2237.	2346.	2117.		41447.
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1928-78 W/O SALMON FALLS, INCLUDES 1939 RIGHTS

IVER DIGITAL MODEL

UPPER SNA

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מבוטנ	FLOW	2061	1229.	1093.	1097.	1183.	1690.	2548.	5784.	4528.	3448.	2344.	3338.	30343.	2833.	2034.	1076.	1108.	1089.	1119.	1468.	2632.	8006.	6071.	4382.	4091.	35909.	2618.	1226.	1085.	1098.	1026.	2865.	914.	15525.	12255.	6292.	5520.	4663.	55088.
	UTFLO	1845	1041	922.	922.	1038.	1495.	2367.	5431.	4313.	3288.	2170.	3202.	28034.	2653.	1871.	922.	955.	958.	967.	1269.	2194.	7664.	5727.	4121.	3810.	33111.	2402.	1041.	922.	922.	863.	2706.	595.	14741.	11806.	6029	5260.	4418.	51736.
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1928-78 W/O SALMON FALLS, INCLUDES 1939 RIGHTS

UPPER SN. RIVER DIGITAL MODEL

RIGHTS																																												
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SNAKE RIVER DIGITAL MODEL	SYSCON S		17637.	20316.	23707.	27240.	30074	20001	32031.	32407.	31049.	28242.	24107.	20940.	17494.	05905.	15427	16725	10746		22506.	25045	27755.	29738.	31249.	33410.	28570.	23074.	18528.	. 291873.	17468	10000	23272	26836	30047		004-00	30971.	39368.	39929.	32597.	24904.	20407.	21230.345170.
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	WALCO EOM	700.	9 8	9 8		9 6	96		951.	951.	952.	952.	700.	9655.	700.	700.	700	700	700	700	920.	951.	951.	952.	952.	.669	9654.	700.	700	700.	700	700.	700.	950.	970.	970.	952.	952.	669		9692.
RIGHTS	LAKE INFLOW	2050.	590.	629.		. 610	258.	2999.	3643.	3485.	4201.	3947.	3098.	25071.	2618.	. 999	272.	193.	157.	241.	1545.	5043.	5809.	5859.	5604.	4764.	32771.	1932.	727.	297.	159.	232.	142.	1869.	12583.	9196.	7494.	7158.	4135.		46525.
1939	LS OUTFLO	1990.	613.	9 6	5 ¦	27.	246.	3102.	3777.	3305.	4191.	3841.	3045.	24823.	2582.	625.	161.	89.	27.	222.	1483.	5063.	5928.	5940.	5607.	4563.	32290.	1857.	664.	210.	30.	28.	30.	1931.	12674.	9893.	7534	7138.	4032.		46022.
INCLUDES	AMERICAN FALLS FLOW EOM OU	2404.	4616.	7569.	10622.	13336.	15876.	14607.	12593.	10840.	8063.	5736.	4357.	10619.	3772.	5647.	8353.	10950.	13297.	15733.	15999.	12703.	10477.	6177.	2330.	<del>-</del>	105439.	168.	2252.	5159.	8237.	10982.	15785.	15798.	17000.	17000	11045	5716.	3463.		112603.
FALLS,	AMERI INFLOW	2311.	2826.	3568.	3123.	2741.	2786.	2009.	2009.	1849.	1905.	1899.	1848.	28874.110619	2084.	2500	2867.	2686.	2374.	2658.	1916.	2006.	4027.	2051.	2070.	2387.	29626. 105439	2065.	2749.	3117.	3107.	2773.	4833	2110	14145	102R2	2179	2291.	1984.		51636.112603
SALMON	BLKFT FLOW	384.	1090.	1837.	1513.	1277.	1285.	297.	307.	297.	307	307	297.	9198.	307.	956.	1261.	1088.	929.	1099.	297.	307.	2392.	307.	307.	777.	10027.	307.	1192.	1565.	1444	1252.	3300	549	10503	BE37		307	297.		31571.
1928-78 W/O	SHELLY FLOW	1520.	1572.	1833.	1547.	1277.	1414.	1248.	2268.	1429.	1423	1146	1210.	17887.	1421.	1368.	1501.	1414.	1277.	1414.	1069.	1747.	4531.	2251.	1782.	2032.	21807.	1550.	1376.	1661.	1557	1388.	3506		14626	10596	. 8000	1919	1565.		43233.
1928-	H. FORK Rexbrg	877.	837.	872.	.669	371.	25.	17.	300.	177.	947	1136	210.	6468.	23.	Ö	557.	540.	474.	602.	446.	968.	957.	554.	608	352.	6080.	573.	773.	697	200	653	725		2364	1004		280.	186.		10148.
DEL	TT RIV OUTFLO	223.	182.	197.	223.	205.	251.	180.	120.	67			17.	1798.	æ	7.	178	167.	164.	204.	327.	244.	314.	. 99	59.	16.	1764.	64	171	190	20.5	503	. 000	. 00.1		1020.		. 6	26.	2	3232.
DIGITAL MO	> >	272.	239.	233.	229.	200.	250.	305.	387	275	224.		174.	2955.	307.	194	188	172.	164.	205.	362.	622	874	473.	335	281.	4177.	256	234	20°						. 400		400.	243.	3	5550.
RIVER DIC		. 689	. 669	760.	546.	359.	228.	511.	438	. A. E.		1076	448.	7569.	286.	377	448	455	405.	504	505	1238	777	669	80 S	557.	7055.	620	6. CRO	. 700		50.4		. 766		2034.	1124	127	997.		8848.
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KE RIVER DIGITAL MODEL

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	H. FORK Ashton	704.	514.	638.	420.		. 200		. 7 . 7		1144.	865	451.	8797.	680.	508.	630.	414.	575.	501.	. 670	106.		1125.			8056.	641.	583	639.	626.	556.	446	4422	1137	0.00	926	855.		9571.	
	OUTFLO AS	281.	115.	246.	31.	53.			401.	446.	718.	436.	3 <del>1</del> .	3165.	270.	117.	246.	94	222.	61.	133.	461.	212.		360.		2835.	247.	218.	246.	246.	222.	. 61.	149.	האט	, R	F 24.	446.	•	4042.	
	PARK EOM OL	681.	.967	770.	946.	1127.	1289.	1350.	1328.	1201.	709.	578.	875.	11651.	. 998	983.	947.	1120.	1079.	1204.	1350.	1259.	1350.	856.	798.	1082.	12894.	1071.	1071.	1057.	101	969.	101	1350.	1350	1330.	. 000	776		13153.	
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)	HENRYS LAKE EOM OUTFLO	664	690.	716.	742.	768.	794.	821.	843.	843.	838.	759.	713.	9191.	727.	745.	765.	786.	808	830.	843.	843.	843.	841.	791.	746.	9568.	763.	780.	810.	831.	843.	843.	843.	843.	843.	843.	783.	735.	9759.	
	HE I SE FLOW	906	1216	1093.	1093.	1030.	830.	1331.	6144.	6671.	5973.	4927.	2829.	35206.	2052.	1222.	1088.	1085.	975.	786.	723.	6299.	6117.	6060.	4861.	4413.	35682.	2110.	1258.	1144	1097.	712.	1081.	4007.	6206.	9587.	6 198.	5960.	3134.	42495.	
	S OUTFLO	4845	104	922.	922.	863.	639	1183.	5694.	6286.	5779.	4716.	2642.	32532.	1845.							5856.			4551.	4	32845.	1845	1041	922	922.	556.	917.	3773.	5808.	9014.	5865.	5697.	2929.	39290.	
0761	ALISADES EOM O	1 904 1		11865	12076.	12285.	12927.	14000.	14000.	14000.	11924.	10382.	10650.	146592.	10433	10843	11249.	11398.	11506.	12110.	13545.	14000.	14000.	12724.	12141.	11313.	145261.	11532	12225	13076	13309.	13780	14000	14000.	14000.	13985.	13816.	11789.	11955.	. 157466.	
MUDEL	" ₃	•		1366	1134.	1071	1282.	2287.	5742.	6354	3777.	3235.	2948.	32357.	1650	1452	1328	1071	941		_	~	5755.	4615.	4035.	3374.	33858.	8000	1734									3739.		40289	
DIGITAL	AKE DIJTELO INFLO			. 0		. 60	3.5	30.	503	2052	1845	1845.	1488.	8408.	080	244	229		80	3.5	30.	772.	343.	1845.	1845.	1488.	7173.	5	2 5	: 3		. K	3 .	30.	489.	1945.	1845.	1845.	1488	8851	
RIVER DI	چ د		5128.	5128.	5364		א נכ	6330	0	0	6	0	4631.	73392.	4597	4507	4597	4834	FOOR .	5182	5478	6958.	8470.	7464	6237.	5213.	68655.	4	5213.	52.5	5460	5684	5854	6413.	7523.	8364.	7851.	6463.	C	74459.	
KE K	JACKSON		262.	140.	. 69.	. 707	276	. 61 4	2705	2150.	. 646	478	298.	8422.	Č		, 6	200	222	185	359	2311	1955	950	7.	527.	8263.								•	•	•	563.		9360.	1
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UPPER KE RIVER DIGITAL MODEL

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		OUTFLO	778.	614.	676.	147.	149.	188.	2579.	4606	1000	. / 1 10	5679.	5511.	2862.	28996.	Ċ	0000	629.	619	136.	195.	195.	2240.	4726.	4274.	5414.	5318.	3254.		27695.	987.	613.	681.	165.	58.	71.	5044	5866.	4619.	E E A A		0440	3440.	32535.	
	WALCO	EOM	700.	700.	700	700	700	810.	970			951.	952.	952.	669	9783.	6	90.	700	700.	700	700	700	950.	951.	951.	952.	952.	669		9654.	700.	700.	700.	700.	700.	700	970	970	970		202	952.	669	9712.	
RIGHTS	LAKE	INFLOW	1192.	614.	.929	147.	149.	298.	2131		6360.	6753.	7499.	7179.	3165.	37163.	!	1085.	625.	619.	136.	195.	195.	2738.	6301.	5597.	7197.	6882.	3953.		35524.	1340.	613.	681.	165	R.	7.1	15 CR	6796	6244	1,44.	1421	7229.	4253.	40382.	
1939		TFLO	1119.	550.	610.	45.	28.	235			6400.	6836.	7529.	7155.	2946.	36554.		887.	497.	462.	33.	27.	160	2735.	6416.	5616.	7192.	6888	3714.		34628.	1163.	321.	485	251	27	. 6	. 00 0			0248	7521.	7305.	4198.	39995.	
INCLUDES	AMERICAN FALLS	EOM	2539.	4930.	8037.	11266.	14318	1200			14575.	11568.	5710.	314.	<del>-</del>	107260.		1666.	4195.	7421.	10679.	13832.	16695.	16448.	14666.	11990.	6472	1426	-	•	105491.	1197	4197	7763	10000	1000	15555	10040			1,000	11122.	5557.	3156.	.125096.	
FALLS.	AMERI	INFLOW	2458.	2942.	3716.	3274	3079		. / 167	3280.	4244.	4175.	2225.	2171.	2741.	37222.107260		2594.	3026.	3688.	3290.	3180.	3024.	2659.	4897.	3299	2244	2269	2431		36601.105491	240	3322	4050	2447		2000	3 100.	0130	7078.	6652.	2257.	2230.	1986.	45347.	
SALMON	BLKFT	FLOW	483.	1313.	2048	1605	. P. C. C. F.		1339.	1649.	2418.	2384.	307.	307.	870.	16243.		690.	1426.	2064.	1617.	1587.	1352.	790.	2914	1444	202		774		15272.	907	4640			. 6661	1138.	1511.	4218.	4782.	4861.	307.	307.	297.	23358.	
1928-78 W/D	HE!!	FLOW	1491	1514	1065	1665	1000	.0/61	1414.	2162.	4525.	4458.	2269.	1921	1702.	26664.		1661.	1502.	1985.	1648.	1575.	1479	1164	4655	3464			1921		25371.	0077	1403		2.43.	184	1277.	15//	4537.	6175.	7106.	2400.	1960.	1350.	33518.	
1928-7	H FORK	EXBRG	1057	000	. 650			. 191	887.	1307.	1788.	1313.	244	910	256.	10559.		974.	919.	1033	793	188	700		. 425F	747		466	324		9747.	000	500		. 707.	980	826.	795.	1271.	1667.	1493.	416.	220.	402.	11195.	
JDEL		OUTFLO	+ 22					224	337.	307.	379.	218	25		84.	2370.		106.	198	212	203	186	300	373	450					133.	2739.		143.	218.	289.	242.	210.	224.	497.	613.	575.	159.	20.	80.	3271.	
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H OUTFLO AS	261.	128.	246.	31.	222.	61.	470.	766.	738	161		461.	446.	4291.	347.	284.	246.	31.	230.	246.	316.	577.	544.	497.	528.	446.	4292.	299.	272.	246.	31.	222.	61.	138.	589.	649.	461.	461			3876.
PARK EOM OL	768.	895.	888.	1090.	1080.	1266.	1350.	1350.	1350		1242.	1098.	995.	13372.	985.	985.	.866	1223.	1257.	1276.	1350.	1350.	1350.	1146.	996	880.	13767.	871.	871.	868.	1061.	1043.	1208.	1350.	1350.	1350.	1187.	105R	. 020	9/6	13196.
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9 RIGHTS
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INCLUDES 1939 RIGHTS
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1928-78 W/O SALMON FALLS.
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MILNER FLOW	905.	171.		. 000		3412.	6/52.	4852.	9641.	204.	200.	50.	27502.	1766.	2198.	4312.	1095.	2159.	3302.	2411.	o į	4623.	500.	200 1	20.	22317.	915.	1317.	708.	294.	225.	233.	2582.	.0707	. 000	3 8	3	20.	14233.
TFLO	1957.	692.	. 00.0	707	211.	3484.	9008	9744.	13901.	5499.	5543.	3571.	54592.	2828.	2737.	4280.	1006.	2119.	3477.	3617.	38 16.	8154.	5665.	5507.	3674.	46881.	1996.	1826.	702.	147.	116.	267.	3811.	6855.	9300	5537	.7766	3544.	39629.
WALCO	700.	. 200	. 60	.00.	933.	970.	910.	970.	970.	970.	952	669	10233.	700.	700.	970.	700	970.	970.	970.	951.	970.	952.	952.	699.	10502.	700.	700.	700.	700.	700.	970.	970.	970.	9/0.	952.	952.	699.	9982.
LAKE INFLOW	2439.	692.	730.	252.	444.	3521.	9382.	11426.	15027.	7406	7777	4404.	63002.	3363.	2738.	4550.	736.	2389.	3477.	3827.	5005.	. 6606	7533.	7256.	4542.	54514.	2574.	1826.	702.	147.	116.	537.	3929.	8120.	10604	7390.	7275.	4372.	47593.
TFLO	2459.	572.	450.	30.	260.	3336.	9221.	11214.	14818.	7410	7331	4350.	61453.	3218.	2649.	4497.	612.	2281.	3451.	3794.	5049.	8850.	7464.	7246.	4553.	53663.	2515.	1708.	541.	122.	27.	485.	3725.	8062.	10557.	7277.	7254.	4226.	46500.
AMERICAN FALLS FLOW EOM OU	2799.	6002.	9613.	14171.	17000.	17000.	17000.	17000.	17000	17000	11704	9627.	156007.	9194.	11951.	13000.	16366.	17000.	17000.	17000.	16808.	17000.	11377.	5719.	2961.	155375.	2638.	4695.	7978.	11348.	14425.	17000.	17000.	17000.	17000.	13960.	8661.	7817.	53609.139523.
AMERI INFLOW	2179.	3776.	4061.	4588.	3089.	3336.	9400.	11496.	15203	. 8808	9696	2458.	70364.1	2904.	5408	5546.	3978.	2915.	3451.	3973.	5109.	9412.	2448.	2076.	1979.	49196.	2267.	3766.	3824.	3491.	3104.	3061.	3903.	8335.	10929.	4855.	2464.	3609.	53609.
BLKFT FLOW	307.	1918.	2280.	2634.	1337.	1402.	7373.	9708	13335			586.	47516.	688	3691	3743.	2146.	1278.	1635.	2007.	3212.	7547.	418.	307.	297.	27169.	307.	2025.	2111.	1600.	1389.	1352.	2127.	6173.	8783.	2983.	494.	1899.	31244.
SHELLY FLOW	1397.	1872.	2228.	2800.	1491.	1553.	7942.	11393	15210			2196.	59006.	1917	372B	3702.	2660.	1547.	1740.	2378.	4443.	9403.	2484.	1906.	1256.	37163.	1559.	2155.							10773.	4976.	2064.	3033.	41398.
H. FORK Rexbrg	861.	1042.	1119.	841.	881.	977.	2532.	2793	2707	. 1017		939.	17152.	1322	1456	1176	904	1045	1132.	1138.	1401.	1901.	577.	274.	779.	13105.	1053	1219.	1123.	821.	930.	922.	905.	2546.	2702.	1283.	742.	1155.	15398.
TT RIV		252.	244.	245.	216.	298.	805			. 600	. 080	163.	4848.	010		267	264	240	238.	341.	354	603.	256.	37.	117.	3228.	10	248	236.	229.	210.	256.	328.	665.	1060.	618.	283.	248.	4532.
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ODEL	SYSCON H. FORK	1895	1927.	1966.	2201.	2221.	2333.	2334.	2345	2330	2068, 394079	1842.3	1692.39347	25155.*	1724.	1918.	2019.	2042.	2039.	2263.	2333.	2345.	2345.	2112.	1918.	1776.	24835.	1808.	1847.	1905.	2152.	2188.	2332.	2333.	2345.	2345.	•	1941.	1757.
SNAKE RIVER DIGITAL MODEL	SYSCON SNAKE	28417	29899	33410.	33876.	36735.	38006.	39672	39807	40238	33531.	26372.	25036.	19514.404999.	24976.	27499.	30996.	34162.	36065.	37182.	37943.	39555.	39968.	34086.	27399.	25177.	19847.395009.	24263.	26721.	30076.	33546.		37293.	38525.	39958.	39958.	32493.	24739.	21691.
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¥-	H. FORK SHTON	739.	697	455.	616.	621.	1478.	1579.	1001.	1049.	959.	860.	10789.	822.	607.	.929	. 199	608	521.	916.	1542.	1229.	1043.	947.	.6/8	10458.	746.	. 169	675.	464.	639.	558.	879.	1657.	1163.	1000	907.	845	10231.
	DUTFLO AS	280.	246	31.	222.	162.	589.	709.	446.	555.	498.	446.	4446.	349.	151.	246.	246.	222.	61.	309	737.	585.	561.	461.	446.	4375.	312.	277.	246.	31.	230.	133.	348.	643.	522.	516.	461.	446.	4166.
	PARK EOM	. 696	909.	1222.	1240.	1350.	1350.	1350.	1335.	1073.	911.	822.	13578.	814.	965.	1042.	1064.	1059.	1281.	1350.	1350.	1350.	1117.	959.	874.	13223.	865.	872.	928.	1174.	1208.	1350.	1350.	1350.	1350.	1136.	1005.	888.	13471.
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TFLO	2188.	1885.	657.		282.	3264			9141.	7606.	7103.	4150.	52413.	2284.	1824.	1345.	30.	1195.	3668.	5895.	8496.	16220.	7563.	7128.	4528.	60176.	5848.	3824	3962	2666	1545	3445	7305	14005.	9992	7622	6630	4881.		71725.
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SALMON FALLS.	GAIN USED BY MINDKA BURLE	ó	ö	o O	Ö	Ö	o O	ö	ö	o O	15145.	5265.	25805.	46215.	o.	ö	ö	Ö	Ö	ö	ö	ö	ö	9100	12545.	17745.	39390.	o.	Ö	o.	o O	ö	o	ö	Ö	ö	7865.	10530.	25025.	43420.
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1928-	RENT	ö	0	Ö	ö	•	o.	Ö	ö	77611.	.394170.	393655.	391916.	* * * * *	o.	ö	Ö	ö	ö	ö		77611.	2345.363090.	2203.394170.	2023.392627.	.389000.	***	Ö	ö	o O	ö	o O	ö	o.	Ö	77611.	394170.	392638.	.386386.	25155.*****
MODEL	SYSCON H. FORK	1934.	1974.	2075.	2149.	2202.	2334.	2335.	2345.	2345.	2307.3		2051.3	26233.****	2066.	2099.	2201.	2281.	2333.	2334.	2334.	2345.	2345.	2203	2023.	1848.	26412.	1846.	1874.	1938.	2003.	2039.	2261.	2344.	2345.	2345.	2225.	2068.	1866.	
	SYSCON	29548.	30970.	32348.	34115.	37198.	38339.	39470.	39217.	39646.	33452.	26293.	22400.	.402996.	21570.	23968.	28061.	33020.	36844.	37759.	38491.	38881.	39678.	33083.	25169.	20154.	.376679.	19783.	23490.	27530.	31911.	35725.	37893.	38419.	39396.	39702.	35394.	27724.	23477.	20458.380443.
RIVER DIGITAL	FRCSTH	o	Ö	ö	4448.	4328.	4113.	4025.	3499.	1957.	587.	Ö	ö	22957.	o	o	ö	3228.	3432.	3247.	2855.	2517.	1992.	568.	o.	ö	17839.	Ö	o	ó	3673.	3733.	3391.	3590.	3225.	1880.	.966	o.	Ö	
SNAKE R	FCSPSN FRCSTH	o.	0	ö	o O	o O	o.	500.	.066	550.	o	0	ö	2040.	o.	Ö	Ö	Ö	Ö	ö	Ö	190.	590.	ö	ö	Ö	780.	Ö	Ö	Ö	o.	o.	Ö	90.	790.	520.				1550.
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N-1	H. FORK ASHTON	826.	752.	700.	692.	654.	619.	1172.	2877			1043.	1022.	948.	12791.	866.	767.	741.	766.	.099	789.	954.	1439.	1369					11103.	728.	7.10			. 000	920.	493	988	1411.	1120.	911.	889.	892.	10105.	
	OUTFLO A	326.	288.	246.	246.	230.	166.	423		1308.		461.	461.	446.	5308.	334.	305.	246.	246.	227.	304.	356.	673				ρ,	446.	4701.	322	201	. 970	240.		222.	9.	337.	591.	501.	461.		446.	4186.	
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SALMON	HEISE FLOW	2873	2881	2760	2295	. 00.7	. 20.	8 14.	4101.	14842.	12277.	5991.	4997.	3545.	58114.	2376	2261.	1152		7007			4333	4807	14097.	6777.	5550.	4398.	47670.	0	2108	1233.	1118.	1089.	691.	1186.	3085.	11986.	10105.	7273.	5295.	3859.	70004	49041.
78 W/O	S DUTFLO	34.90	2622	25.44		, and a		615.	3672.	14019.	11698.	5700.	4724.	3292.	54130.	2069	1969	000	3464.	7 0 0 0 0 0 0 0 0			2007	4263.	13656.	6209	5283.	4091.	44163.	•	1845.	1041.	922.	922.	555.	1002.	2787.	11434.	9787.	6976.	5110.	3643.		40023
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ODEL	PA NFĽOW	*****		1011		1480.	1269.	1330.	3795.	13674.	12044.	5794.	-	3422.	53836.	1878	15.44					1207			-	6648.	4256.	3194.	42765.		1789.			1293.	1128.	1256.	2765.	10983.		7442	4287	3293.		4/511.
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KE RIVER DIGITAL MODEL	JACKSON LAKE LOW EOM OU	9	6048.	6048.	6048.	6397.	6804.	7200.	7955.	8110.	8231.	7838.	6659	5529.	82868.	200	מ נ		2229.	5910.		. /689	6802	8288.	8221.	7852.	.6099	5453.	78290.		5453.	5453.	5453.	5812.	6091	6357	68 15.	8171	8039	A201		6079.		79196.
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RIGHTS	LAKE	INFLOW	4464.	3777.	3933.	2903.	1080.	3615.	6986.	17440	4040.	1035.	7798.	7632.	4969.	75651.	2704	100	727		194	166.	3251.	4399.	6116.	12281.	7954.	7387	4070	0 t 0 t 0 t 0 t 0 t 0 t 0 t 0 t 0 t 0 t	51994.	2424.	621.	722	221.	6	1816	. 46.76		7000	7083.	7534.	7334.	4775.	47594	
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INCLUDE	AMERICAN FALLS	EOM	9000	11000.	13000.	15000.	17000.	17000.	17000	1000		7,000	11197.	5562.	2707.	152466.	2415		9220.	8042	13975.	17000.	17000.	17000.	15882.	17000.	107 10	5134	1 7 0 0	22.15.	132194.	2325	5364	8775	12409		44200	36			1,000.	12325.	6678.	3819.	40000	. 330082
FALLS,	AMERI	INFLOW	3991.	5719.	5852.	4855.	3036.	3622	7214	17666		113/1.	2436.	2471.	2225.	70457.	2442		4622.	4037	5363.	3077.	3123.	4371.	4912.	13590.	2176	2266		2080.	52030.	2552	3577		3664	9004		3408.	4822.	10597.	7377.	3450.	2174.	2021.		50033.
SALMON	RIKFT	FLOW	1945	4040	3969.	3023	1276.	1609			15/21.	9550.	307.	307.	297.	47264.	702		2883.	2294.	3522.	1585.	1544.	2623.	3094	11792	202			297.	30555.	2			2010		1040.	. 61.18	2983.	8909	5780.	1542.	307.	297.	,	29034.
1928-78 W/O	CHELLY	FLOW	2704	3882	3902	3032	1330	1647		2014.	16804	11197.	2215.	1942.	1476.	55742.	7007	1024	2885	2235.	3477.	1616.	1650.	3112.	4216.	13472	2288			1508.	40104.	4573	1000	1999	2166.		1507.	1/46.	3707.	11106.	8063.	3521.	2131.	1548.		40945
1928-	מטנו	REXBRG	1303	1405	1250	1170	1053			2116.	4540.	2713.	821.	723.	908	19199.	1	1043.	1211.	1168.	1345.	1072.	1125.	1128.	1475	2674				548.	13754.	0,00			1 40.	1140.	1049.	850.	1278.	2114.	1508	771.	501.	756.		13311.
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	OUTFLO	2165.	2081.	1563.				3158.	4441.	5783.	5667.	5574.	3979.	36791.	1107.	1793.	. 169	119.	. 999	3388.	6874.	12664.	13627.	5786.	5256.	3610.	55586.	2240.	2165.	1654.	930.	11/5.	3446.	3/41.	13167.	12041.	5946	5466.	3684.	55655.
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	TFLO	2503.	2122.	1491.	51.	27.	2389.	3340.	5856.	7265.	7471.	7223.	4766.	44503.	1517.	1730.	553.	44.	825.	3241.	7593.	14240.	15047.	7663.	6901.	4281.	63634.	2561.	2065.	1503.	807.	1220.	3330.	4011.	14380.	13604.	7872.	7205.	4511.	63070.
AN FALL	INFLOW EOM OU	3965.	6424.	9746.	13834.	16443.	17000.	17000.	16555.	17000.	11214.	5738.	2725.	137644.	3562.	5733.	9334.	15000.	17000.	17000.	17000.	17000.	17000.	11040.	5696.	3511.	138876.	3883.	7126.	11093.	14962.	17000.	1000	17000.	17000.	17000.	11407.	5847.	3097.	. 142415.
AMEDI	INFLOW	2735.	4581.	4813.	4140.	2636.	2946.	3519.	5679.	8104.	2321.	2257.	1951.	45680.1	2433.	3905.	4153.	5710.	2825.	3241.	7771.	14531.	15438.	2330.	2043.	2301.	66678.1	3017.	5308.	5471.	4676.	3258.	3330.	4 189.	14621.	13995.	2890.	2153.	1956.	64864.
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> 1 15H	FLOW	1757.	2857.	3052.	2594.	1337.	1416.	1766.	5437.	8398.	2462.	2137.	1379.	34590.	1548.	2216.	2395.	3827.	1424	1577.	6478.	15108.	16286.	2702.	2032.	1992.	57585.	2157	3588.	3782.	3011.	1665.	1815.	2751.	12918.	14021.	3204.	2290.	1580.	52782.
	REXBRG	1091.	1174.	1152.	1114.	935.	936.	1114.	1556.	1604.	607	368	662.	12313.	1043.	1255	1366.	1198.	995.	1052.	1825.	2950.	2281	791.	742.	1059.	16557.	1001	1395	1237.	846.	1091	1094.	1110.	3698.	2937.	1045.	750.	1042.	17447.
	OUTFLO	135.	224.	222.	216.	185.	197.	275.	246.	276.	99	20.00	50.	2147.	116.	929	334	240.	180	295.	489.	953.	760	200.	144.	123.	4063.	8	256	246.	225.	267.	318.	349.	799.	1087.	436.	128.	164.	4464.
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PA	INFLOW	2043	4740.	1766	125	260	149	. 686	10000	10300.	8349.	4645.	3591.	3021.	40911.	11.	1434	1 T	1180	1017	1157	2350	4756	1 1909	5804	4 105	3177.	39955.		. 1677	1646.	1477.	1221.	1103.	1271.	2835.	5737.	64	4623.	· ·	2859.	35632	
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WALCO	EOM	700.	700.	9 6		. 6	.076	970.	9/0.	951.	952.	952.	. 669	10233.	700.	700.	700.	700	700.	970.	950.	951.	951.	952.	952.	669	9925.	700.	700.	700.	700	700.	970.	970.	951.	951.	952.	952.	699.	9944.
LAKE	INFLOW	2537.	2097.	1584.	152.	1126.	30/4.	4280.	10411.	6460.	7699.	7168.	4116.	50704.	1861.	543.	671.	122.	92.	1969.	3311.	5687.	6567.	7789.	6972.	3590.	39173.	829.	1954.	744.	160.	.06	2583.	5834.	6678.	6787.	7938.	6975.	4697.	45269.
	IFLO	2416.	1980.	1511.	. 26	993.	3008	4149.	10388.	6379.	7643.	7149.	3943.	49617.	1705.	466.	. 899	30.	29.	1957.	3245.	5679.	6514.	7768.	7051.	3392.	38503.	760.	1884.	716.	114.	28.	2534.	5761.	6664.	6835.	7911.	6997.	4520.	44724.
AMERICAN FALLS	EOM	3515.	6376.	10247.	14711.	17000.	17000.	17000.	17000.	16391.	10129.	4727.	2547.	. 136643.	3097.	5994.	9297.	12668.	15996.	17000.	16497.	14141.	15968.	9702.	4064.	2351.	126775.	4793.	6787.	9770.	12919.	15940.	17000.	17000.	14212.	10536.	4114.	ю	o.	113074.
AMERI	INFLOW	2915.	4842.	5382.	4521.	3282.	3009.	4327.	10672.	6149.	1991.	2219.	1950.	51259.1	2330.	3363.	3970.	3401.	3357.	2961.	2914.	3577.	8699.	2105.	1877.	1860.	40414.	3285.	3879.	3699.	3262.	3050.	3594.	5939.	4130.	3504.	2035.	3283.	4635.	44296.113074
BLKFT		881.	3114.	3614.	2817.	1663.	1346.	2517.	8916.	4282.	307.	307.	297.	30061.	307.	1676.	2134.	1696.	1832.	1418.	923.	1693.	7093.	307.	307.	297.	19683.	1243.	2218.	2035.	1525.	1397.	1929.	3956.	2196.	1569.	307.	1513.	3036.	22925.
SHELLY	FLOW	1961.	3150.	3568.	2820.	1662.	1415.	2601.	10662.	6140.	2366.	1846.	1341.	39532.	1478.	1622.	2109.	1822.	1873.	1443.	1316.	3239.	9253.	2485.	2210.	1501.	30351.	2000	2233.	2034	1663.	1547.	1991	4240.	3716.	3761.	2775.	3304.	4255.	33520.
H. FORK	REXBRG	1213.	1232.	1224.	1114.	1074.	860.	1214.	2573.	1390.	350.	384.	883.	13512.	1045.	931.	1074.	836.	963.	850.	1015.	1056.	1685.	504	391.	814.	11164.	1383	1274	1073	840.	1007	1245.	1460.	1268.	1309.	224.	275.	233.	11591.
7181	OUTFLO	183.	250.	261.	248.	246.	263.	340.	854.	349.	18	54	89.	3155.	105.	224.	235.	228.	192.	291.	426.	135.	502.	87.	32.	114.	2571.	181	218	222	191	18.5	303	443.	259.	166.	<b>8</b>	18.	17.	2201.
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RIRIE EOM (	500.	514.	200.	517.	500.	546.	754.	900.	900	900	.006	.006	500.	513.	200	523.	200	536.	. 199	828.	900	900.	900	900	500.	510.	500.	504.	500.	549.	718.	858.	900	900	900	899.
INFLOW	24.	29.	31.	35.	50.	61.	224.	311.	71.	21.	22.	<b>18</b>	22.	28.	32.	38.	31.	51.	147.	177.	110.	37.	17.	<del>1</del> 9.	25.	25.	22.	<del>1</del>	20.	64.	185.	156.	<del>1</del> 01.	30.	50.	15.
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SYSCON	H. FORK	1028.	1183.	1405.	1610.	1807.	1978.	2201.2	2210.2	2103.1	843.1	116.	116.	16600.743207	153.	311.	528.	748.	966	1156.	1527.	1768.	1869.	1764.	1609.	1430.	13829.	1447.	1471.	1523.	1755.	1777.	1991.	2072.	2149.	2221.	1804	m.	1821.	21548.
SYSCON	SNAKE	10172.	13797.	17239.	20303.	23628.	27217.	28525.	26950.	24508.	14574.	8 196.	8089	. 223199.	9845.	13466.	17253.	20523.	25755.	29359.	34709.	39419.	39618.	33309.	25808.	20941.	310005.	20651.	22904.	26514.	30040.	34972.	37425.	37815.	40060.	39822.	30557.	22193.	18864.	15427.361818.
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GRASSY EOM OU	•	i		<del>1</del> 3.	17.	21.	25.			. 70	89.	14.	c	; <del>-</del>	<u>:</u>	285.	4	-	<del>1</del> 8.	23.	29.	34	43	- 0	. 5		114.	121.	. 86		677.	. 86	<del>1</del> 00	101.	102.	104.	106.	112.	136.	151.	152			5	1320.	
H. FORK ASHTON	t	. 777	556.	453.	429.	403	497			1156.	906	1881	1461	100	. 671	9783.	731.	533.	418.	416.	428.	486	, CC &		1009.	. 104	1001	988.	929.	1	9575.	771.	736.	653.	448.	650.	503.	782.	1360.	1026	100				9657.	
H OUTFLO AS		298.	127.	3 <del>1</del> .	31.	28	. +		90.	461.	446.	1303	. 4		348	4298.	324.	123.	31.	31.	28.				628.	4 30.	461.	461.	446.		3144.	291.	274.	246.	31.	222.	61.	275.	524.	446			486.	ċ	3564.	
PARK EOM OU		293.	419.	612.	790.	090			1328.	1305.	1171.	275		. 60	94.	8525.	93.	216.	404	589.	768				1350.	1350.	1199.	1077.	910.		10092.	901.	901.	928.	1138.	1139.	1330	1350	1350.	1001	. 000	999	731.	1031.	13018.	
ISLAND SEEPAG		·	ö	Ö	c	; c	<i>.</i>	·	ö	o.	c		<i>.</i>		O	ö	ö	Ö	Ö	ó	i c				o.	o O	ö	Ö	o O		Ö	Ö	Ö	Ó	o	Ċ	c	i c	ċc	; c		•	o ·	o.	o.	
INFLOW SE		298.	253.	223.	500		198.	211.	289.	459.	243		. 020	7.16.	375.	4204.	324.	246.	219	216	202		203	383	774.	522.	353.	375.	299.		4123.	291.	274	273	241	223	25.2	. 900	300°.		44 (	324.	350.	318.	3848	) )
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HENRYS LAKE EOM OUTFLO		731.	755	780			826.	843.	843.	843			454	47.	22.	7790.	57.	S.	101	. 96.		. 60	211.	258.	337.	415.	450.	411.	422.	!	3059.	877	770.	707	ער ער			. 656	900	. 600	748.	754.	697.	726.	7211	
HE I SE FLOW		2059.	1287			1180.	1072.	944.	895.	4745			7273.	8520.	2366.	37385.	1570	1265		. 464.		1142.	857.	1165.	7264.	12023.	6621.	5202.	3753.		42931.	9000	2433	1100	1146			1323.	2004	66/0	12417.	7893.	5467.	3282.	ATENA	
S OUTFLO		1724.	1 7 0		. 000	983.	833.	642.	595.	4237		. 60/6	6947.	8278.	2039.	33759.	1216			. 500	924.	833.	615.	595.	6489.	11676.	6356.	4947	3396.		38844.	107				922.	1211.	1311.	1770.	0101.	11703.	7469.	5045.	3008.	42860	400004
PALISADES EOM O		4317	0		0.047	5434.	5441.	5944.	7146.	9678		9851.	7316.	3125.	4767.	73251.	9	100 E		2010	9030	6518.	7128.	11415.	13447.	13513.	14000	13479	13275.		115037.	07707	16446.	12955	13042.	13/30.	14000	14000	-		13708.	11820.	10448.	10899.	007097	. 156469.
PA		1861			1 144	1070.	840.	1145.	1816			5984.	4465.	4125.	3701.	34560.	4000			1220.	1096.	1301.	1225.	4905.	8566.	11808.	6920	) 4	3237	4	47693.	0,00	2040.	1,03	- •	_	_			6153.	11534.	5654.	3735.	3497.		41531.
TELO		242		240.		3.	28.	31.		י נ	. 7017	567.	2460.	2460.	2083.	10359.	Č					28.	31.	29.	83.	3405	. α	3 a	1488	r	8875.	0	296.	240.	240.		28.	31.	30.	30.	3480.	œ	1845		1	9582.
JACKSON LAKE	<u> </u>	ĸ		4654	4851.	5037.	5384.	5629	6010		. 7610	8346.	က	4	4	63935.	0	2000	3308.	36/8	3965	4295.	4589.	5418.	8172.	8206	7697	627		0	63864.		5184.	5184.					6180.	8354.	8271.	7402.	5956	84	1	. 73538.
JACK	TIME FOR		244.				375.			•		28	9	က	388.	8657.	i i	. 000		400	318.	358.	325.	891.	2898.	3540	"	7 4	040	303.	12073.		331.					204.		2266.		_	503		1	9755.
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UPPER SNAKE RIVER DIGITAL MODEL

MILNER	#01L	183.	188.	277.	287.	275.	188.	ċ	o	0	200.	000		20.	1848.	180.	221.	285.	308.	245.	1013.	1558	1570.	2919.	200.	200.	20.	8749.	807.	1259	926	297	286	779	2746	1854	6040				20.	15448.
	5	1340.	328.	411.	265.	134.	164.	2406.	4652.	4834.	6043		. 7776	2730.	28529.	1050.	293.	187.	328.	163.	963.	3775.	6264.	7670.	6085.	5762.	4319.	36859.	1931	1843	207	140	144	000	4324	65.00	4075	62.40		22.00	3882.	42896.
WALCO	T C	700.	700.	700.	700.	700.	700.	950.	951.	951.	952		0	. 669	9654.	700.	700.	700.	700.	700	970.	970.	970.	970.	952.	952.	669	9982.	200	202	9 6	. 6	. 6	. 6		070		970	906	. 706	669	9982.
LAKE	INFLOW	1647.	328.	411.	265.	134.	164.	2916.	6401.	6464	7880	. 0007	0294	2820.	36025.	1247.	293.	187.	328.	163.	1233.	4119.	7539.	9139.	8044.	7475.	5233.	45000.	2345							101		11901.	9 6 6	192.	4573.	50585.
i	OUTFLO	1562.	297.	331.	137.	27.	104.	2901.	6321.	6497		9050	6515.	2526.	35247.	1096.	220.	95.	196.	27.	1218.	4241.	7421.	9250.	8171.	7389.	5160.	44484.	2243		. 020	. 60				4030	. 1049.	. ראארו	8285.	7196.	4424.	49960.
CAN FALLS	EOM	701.	3763.	6542.	9334.	12292.	15105.	14751.	10348	549B	•		o o	o O	78335.	1346.	4334.	7197.	10012.	14354.	17000.	17000.	17000.	17000.	10711.	5058.	1584.	122596.	46.00	1020	4209.	7288.	. 6444	14770	966	1,000	1,000	200	104/9.	4942.	2272.	52763.123937.
AMERICAN	INFLOW	2304.	3360.	3110.	2929.	2985.	2917.	2706.	2153	. 606		2989.	6812.	2621.	36796.	2483.	3208.	2958.	3011.	4369.	3864.	4416.	7702.	9630.	2508.	2207.	1877.	48233.		45.44	4003.	3635		4405.	3408.	4/14.	. 500	12245.	23/9	2130.	1933.	52763.
BLKFT	FLOW	307.	1641.	1419.	1350.	1553.	1358.	1046.	307	. 700	. 787	1290.	5393.	975.	16937.	499.	1559.	1390.	1425.	2262.	1746.	2520.	5691.	7513.	334.	307.	297.	25543.	,		2889.	1940.	1423.	2585.	1934.	2721.	6118.	10258.	307.	307.	297.	31185.
SHELLY	FLOW	1481	1621.	1414.	1414.	1610.	1414.	1304	1077		2208.	3366.	6937.	1632.	26379.	1294.	1586.	1414.	1414.	2138.	1680.	2486.	7266.	9276.	2504.	1835.	1683.	34576.		1420	2987	2017.	1529.	2664.	19/4.	2875.	6990.	11200.	2564.	1826.	1299.	39345.
H. FORK	REXBRG	822	957.	754.	791.	794.	821.		. 7007		. 20.0	872.	1338.	796.	10475.	1096.	1016.	782.	726.	1259.	1071.	1931.	2416.	1750.	870.	536.	854.	14306.		1087	1185.	1096.	832.	1351.	802.	825.	2194.	2297.	387.	383.	629	13068.
r RIV		131	218.	205.	186.	214	341	278	. 6	. 40.	126.	- <del>1</del> 8	- <del>1</del> 8	86.	2061.	120.	210.	204.	187.	496.	320.	702.	613.	442.	254.	26.	50.	3624.		147.	231.	238.	209.	322.	271.	228.	483.	584.	18.	<del>-</del>	159.	2901.
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H. FORK		775.	691.	635.	651.	611.	200	675.	1696.	1817.	1017.	987.	919.		10976.	769.	761.	744.	717.	622.	576.	1173.	1941.	1515.	1084.	1078.	968.		11949.	892.	908	776.	783.	705.	802.	1012.	1283.	943.	1217.	801.	478.	10601.
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PARK	E O L	1021.	1021.	1029.	1042.	1044.	1233.	1350.	1350.	1350.	1254.	1137	1040.		13870.	1030.	1030.	1092.	1190.	1221.	1350.	1350.	1350.	1350.	1332.	1262.	1200.		14755.	1189.	1189.	1300.	1350.	1350.	1350.	1350.	1350.	1238.	761.	782.	1146.	14354.
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RIGHTS	LAKE INFLOW	2599.	655.	634.	139.	154.	215.	3495.	9160.	15577.	9339		5053.		53720.	2682.	1847.	721.	126.	974.	3500.	8050.	11580.	16219.	7904.	6841.	4600.	65044.	4925	3282.	2897.	3763.	1448.	3621.	4720.	6929.	6279	7833.	7117	4454		57296.	
1939	TFLO	2517.	550.	468.	30.	28.	30.	3540.	9258	15551	8431		5127.		53373.	2592.	1694.	296.	51.	967.	3369.	8067.	11401.	16078.	7878.	6706.	4616.	63715.	4908	3240.	2807	3609	1343.	3508	4671	6790	6239	7782	7173			56561.	
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OUTFLO A		295.	285.	246.	246.	222.	61.	284.	2		967.	489.	461.	446.	4820.	364.	317.	246	253	. 000		. 4 . 6	352.	654.	518.	461.	461.	446.		4686.	313.	279.	246.	246.	222.	246.	496	1125			. / [0	613.	446.	5492.	
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7	INFLOW	2286.	432.	205.	166.	134.	437.	2747.	6111.	13912.	8123.	7695.	5595.	47843.	1	.070	2000.	1550.	207.	.699	3422.	3149.	6876.	9802.	8190.	5808.	4375.	9	48618.	3762.	3140.	1746.	3609.	1734.	3820.	7623.	12187.	8440.	7779.	7223.	4129.	65192.
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	HEISE	FLOW	2907.	2646.	2390.	2101.	781.	879.	967.	8589	16723	0460	6460.	4700	-	57300.	2604.	2927.	2766.	2305	776	911	3872	14495	22876	10607	6694	) O	1	75842.	3623.	3238.	3108.	2447.	810.	1696.	4478.	15844.	21308.	9152.	6783.	5476.	77963.	
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0 7 0	ALISADES	EOMO	14000.	13400.	12800.	12200.	12679.	13180.	14000.	13545		13498.	14000.	14000	4000	61301.	14000	13400	12800	12200	12013	13658	12228	13230.	12000	14900.	3	14000		160076.	14000.	13400.	12800.	12200	13045	13920.	13370.	12943.	13108.	14000.	14000	14000.	160785	•
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E JALISIA	ш	UTFLO I	217.	207.	291.	31.	28.	31.				3807.	1845.	1845.	1488.	9848.	27.4	341	436				5 6	. אם כשל דער אינו	1020	5201.	2043	1488		13272.	417	326.	449	~	60	9	29.	614	54		8	1488.	13954	ט ט
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K H	JACKSON LAKE	INFLOW	217.	207.	291.	420.	251.	274	305		. 1001	4467.	1451.	479.	430.	10689.	908	344		. 436		2000	מים	. 2000	200	5405.	מ מ	887.		15031.	453	326	449	447	374	440.	531	2647	5072	1700	797	606.	13843	13046.
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	JTT OUTFLO	3138.	3564.	3707.	3203.	1375.	3523.	4024.	13744.	18716.	11549.	11230.	6868.	84640.	6344.	4863.	4450.	3721.	1500.	3766.	9238.	22008.	25952.	11976.	11620.	7812.	.113250.	14229.	5968.	4751.	3747.	2177.	5375.	9446.	18092.	24890.	11724.	10837.	7127.	11332.118362.
	EOM OU	700.	910.	970.	970.	970.	970.	970.	970.	970.	952.	952.	. 669	11062.	970.	970.	970.	970.	970.	970.	970.	970.	970.	952.	952.	. 669	11332.	970	970.	970.	970.	970.	970.	970.	970.	970.	952.	952.	669	11332.
RIGHTS	LAKE INFLOW	3532.	3834.	3707.	3203.	1375.	3523.	4428.	15039.	20142.	13546.	13061.	7502.	92892.	7095.	4863.	4450.	3721.	1500.	3766.	9490.	23266.	27522.	14057.	13459.	8582.	121772.	14965	5968	4751.	3747.	2177.	5375.	10015.	19868.	26391.	13692.	12536.	7747.	127231.
S 1939	LS OUTFLO	3757.	3721.	3504.	2823.	1116.	3409.	4553.	16066.	21821.	15443.	14845.	8156.	99214.	7406.	4685.	4137.	3327.	1391.	3618.	9641.	24196.	28872.	15878.	15105.	9361.	127618.	4 1 0 4	I B B C	4467.	3478	1996.	4895.	9892.	20742.	27051.	14939.	13828.	8201.	130563.
INCLUDES	AMERICAN FALLS Flow Eom Ou	8253.	11000.	13000.	15000.	17000.	17000.	17000.	17000.	17000.	14386.	9480.	10607.	166725.	9000	11000.	13000.	15000.	17000.	17000.	17000.	17000.	17000.	16816.	13621.	16043.	. 179480. 127618. 121772	6	1000	13000	15000.	17000.	17000	17000.	17000.	17000.	14386.	11941.	13782.	173110.
FALLS,	AMERI Inflow	6272.	6469.	5504.	4823.	3116.	3409.	4728.	16339.	22249.	13479.	10484	9535.	55518.106407.166725	5928	6685.	6137.	5327.	3391.	3618.	9814.	24480.	29275.	16370.	12484.	12053.	135562.	000	7887	6467	5478	3996.	4895	10070.	21040.	27453.	12972.	11938.	10303.	47896.120075.108185.130782.173110.130563.127231.
SALMON	BLKFT FLOW	2449.	3776.	3557.	2990.	1659.	1652.	1429.	10023.	15317.	5867	2848	3951.	55518.	3851.	4874.	4345.	3366.	1751.	1871.	7677.	22111.	27201.	14377.	10412.	10127.	38019, 124010, 111963, 135562	7773	. 600	4636	3579	2239	32.18	8816.	18941.	25448.	11089.	9797	8371.	108185.
1928-78 W/O	SHELLY FLOW	3423.	3746.	3482.	2973.	1671.	1690.	1429.	10567	17785.	7419	4320	4752.	63257.	5205	4835.	4307	3348	1809.	1819.	7259.	23514	30009	17638.	13025.	11242.	124010.	600	. 1000 EAR	4534	3423	1918	0000	9028	22051.	28072.	13849.	11910	9825.	120075.
1928-	H. FORK Rexbrg	1815.	1518.	1242.	897.	1060.	1137.	1075.	3344	5430	3305	2553	1757.	25134.	3100	1993.	1340.	1222	1132	1346.	1708	5765	7233	5225.	4098	3857.	38019.	1		1673	15/42	1556	1875	1927	7590	8151.	6846	5320	5467.	47896.
MODEL	TT RIV OUTFLO	245.	258.	268.	249.	215.	239.	339.	883	1616	924	433	322.	5991.	273	281.	276	293	257	380	585	1300	2098	1194	590	488.	8014.	i c		. 000	244	27.6	E 27	649	1566	1875.	892	0	451.	8421.
DIGITAL M	TT RIV STANTY	424.	317.	281.	254.	216.	240.	374	1150	2263	1454		547.	8331.	383	329	287	297	. 86.	38.1	601	1669	25	1788	2 5	728.	10454.		. 603	. 000	3.40	270		711	2033	2468	1431	964	687.	10874.
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	SYSCON SNAKE	29216.	30649.	32016.	33772.	36769.	37810.	38611.	39998.	39847.	33711.	26930.	26917.	7858.406244.	28061.	29521.	30922.	32743.	35653.	37092.	37779.	38758.	39202.	38184.	33938.	31416.	23849.413269.	29369.	30787.	32169.	33918.	36864.	37856.	38603.	38798.	39053.	38240.	30921.	26713.	21896.413289.	
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H. FORK ASHTON	962	944	784			. 607		933.	1983.	1257.	1058				12357.	886.	894.	756.	759.	706.	799	1080.		.2002	1832.	1163.	1152.	1051.	13813.		1019.	897.	786.	842.	820.	.968	875.	1867.	2360.	1275.	1143	. 976		13755.	
OUTFLO AS	7 + 7	305	. 970			910	339.	322.	736.	460.	461			440.	4925.	347.	370.	246.	246.	222	246			921.	805.	461.	461.	446.	8812		396.	317.	246.	246.	303.	381.	346.	703.	936.	515	461		4	5297.	
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HEISE FLOW		3581.	3048.	2791.	2372.	730.	844.	2310.	8979		.0010	6807.	5548.	4364.	51890.	2139.	3045	2500			. 191	899.	4291.	11798.	21393.	8108.	5077.	3762.		<b>66062</b> .	2760	2853	2694	2376				6363	+ FOR FR	44700	11/90.	5624	4055.	57707.	
S OUTFLO		2906.	2762.	2551.	2138.	555.	615.	1759.	7069		8040.	4671.	3663.	3321.	40656.	1845	2726	. 00.00			222	615.	3753.	10680.	20611.	7407.	4502	3348.		. / / £09	2107	2396	2208	1025	ער ה	. 4		. E & 2	44725		11024	4959.	3555.	50721.	
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PA INFLOW		2935.	2162.	1951.	1538.	1318.	1369.	2074	6970		8635	4973.	3735.	3367.	41026.	4720	, ,		.0871	1396.	1185.	1526.	3583.	10374.	20846.	8188	4574	3394		60742.	2125	1706	, ,	. 100		. 6611	- 1	- <	7	14015.	11992.	5031.	3601.	51087.	
TFLO		501.	359.	365.	31.	28.	31	. 6	3 -		2550.	8	1845.	1488.	9512.	č		400	. 905 	31.	28.	31.	29.	737.	5988	α,	) Œ	1488		12876.	900	366		. 6 / 9 .	-	. 70	. e.	. 62		3832.	37	4	1488.	10613.	
JACKSON LAKE		5716.	5716.	5716.	6040.	6306.	655			8340.	8293.	7182.	5635.	4483.	76790.	4714		4/14	4/14	5075.	5384.	5832.	6401.	8033.		5	9	5869	•	74192.	0000			2869.	9202.	6520.	. /289	121	8 103	80/1.	8470.	7304.	6099	82330.	: : :
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1928-78 W/D SALMON FALLS, INCLUDES 1939 RIGHTS

¥-	MILNER FLOW	10235.	4.002.	46/0.	3/20.	1806.	4274.	3968.	2008.	o o	200.	200	50.	35498.	000		4 709.	4205.	3250.	1343.	4033.	5944	6583.	14141.	200.	200	20.	47675.	3323.	3645.	4236.	3223.	1762.	3742.	2986.	9158.	7937.	200.	500.	20.	40462	
	TT OUTFLO	12316.	5294.	42/6.	3318.	1647.	4354.	6935.	12050.	9898.	11428.	10589	6899.	89003.		5169.	5633	4 106.	3261.	1395.	3719.	8019.	15949.	23767.	10830.	10033.	7019.	98900.	6113.	4786.	4230.	3487.	1904.	4024.	4376.	14644.	17882.	12082.	11184.	8164.	97876	
	WALCO	970.	9/0.	970.	970.	970.	970.	970.	970.	951.	952.	050	699.	11314.		970.	9/0.	970.	970.	970.	970.	970.	970.	970.	952.	952.	.669	11332.	970.	970.	970.	970.	970.	970.	970.	970.	970.	952.	952.	669	11332	
RIGHTS	LAKE INFLOW	12979.	5294.	4276.	3318.	1647.	4354.	7222.	13644.	11504.	13338	12240	7507.	97302.		5/29.	5633.	4 106.	3261.	1395.	3719.	8371.	17723.	25466.	12765.	11632.	7842.	. 107642.	6837.	4786.	4230.	3487.	1904.	4024.	4475.	15385.	19486.	14036.	12725.	9037.	2443	1004 14.
1939	S UTFLO	12837.	4823.	3863.	2814.	1100.	3780.	6931.	14478.	12622	14621	12277	7835.	99081.		5632.	5338.	3877.	2803.	941.	3487.	8297.	19162.	27125.	14582.	13096.	8875.	113215.	7016.	4614.	3819.	2910.	1616.	3724.	4011.	14635.	19752.	15139.	13847.	9859.	64004	100942
INCLUDES	AMERICAN FALL Flow Eom o	9000	1000	13000.	15000.	17000.	17000.	17000.	17000	16882	11629		7534.	. 158440.		9000	1000	13000.	15000.	17000.	17000.	17000.	17000.	17000.	15127.	12076.	10744.	170948.113215	9000	11000.	13000.	15000.	17000.	17000.	17000.	17000.	17000.	14870.	8718	5714.	10000	983/3.162302.100942.1004.12
FALLS.	AMERI Inflow	8169.	6823.	5863.	4814.	3100.	3780.	7104.	14761	12912	7000		9198.	95161.1		7196.	7338.	5877.	4803.	2941.	3487.	8477.	19458.	27537.	13369.	10610.	7814.	18907.	5415.	6614.	5819.	4910.	3616.	3724.	4183.	14889.	20184.	13686.	8241	7093.	7	983/3.
SALMON	BLKFT FLOW	6315.	5219.	4453.	3188.	1716.	1965.	5450.	13093	11021	0 105		7258.	74780.		5061.	5314.	3979.	2886.	1457.	1473.	6763.	17171.	25679.	11690.	8778.	5997.	96248.1	2934.	4692.	3967.	3076.	1762.	1675.	2307.	12119.	17445.	11297.	5743	4577.	1	/1593.
928-78 W/O	SHELLY FLOW	6832.	5048.	4312.	3185.	1814.	1860.	5321.	15649	14711	11460		9/2/. 8790.	88716.		5699.	5217.	4075.	3089.	1587.	1754.	7502.	19939.	29552.	14801.	11113.	8055.	112383.	4642.	4561.	4027.	3203.	1908	1812.	2610.	12733.	20958.	15149	8421	6832.	i C	86855.
1928-	H. FORK Rexbrg	4126.	2349.	1313.	1396.	1270.	1299.	1362.	7601		1000	1024.	5163.	49384.		5335.	2797.	1406.	1251.	1109.	1444.	3330.	13944.	17889.	16402.	13908.	11445.	90259.	4984	2595.	1445.	1195.	1247	1347	1725.	6432.	14022.	14068	11079	9068	. 6	69209.
MODEL	TT RIV OUTFLO	339.	354.	310.	303.	261.	314.	762		936	. 009	. 665	345.	6131.		1316.	632.	286.	249.	200.	367.	486.	2806.	4899.	3981.	2908.	2655.	20785.	746	408	295.	233.	227	274	309	743.	2707.	29.15	1702	1289.		11848.
GITAL	TT RIV STANTY	550.	440.	317.	307.	261.	315.	787	מיני מיני	1001		1004.	601.	8549.		1465.	705.	296.	254.	201.	368.	503.	C	62	23	~	2956.	23335.	961	461	305.	238.	228.	275	320.	904		ш.	, –	1592.		14293.
RIVER DI	H.FORK Stanty	2548.	1667.	1018.	1090.	992.	1060.	86.1			40.00	3550.	3042. 2711.	27597.		64	1309.	940.	973.	862.	1021.	1355.	20	35	30	65	2403.	26037.	2106	1325	1045.	1043.	1068	1141	934	2762	6082	4465	2932	2783.		27688.
KE	FL RIV CHESTR	29	7	~	42	84	9.5	5 5	, (	- 6	700	814	1804.	17846.		ຄ	46	351	292	259	31	480	2324	3313	176	129	1109.	12510.	Œ		37	38	3.5	3 6	3 -	147	498	PEP	26.0	2329.		19506.
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OUTFLO		456.	15.	81.	15.	76.	15.	15.	420.	15.	48.	28.	32.	436.	15.		15.	65.	15.	106.	836.	15.	15.	15.	15.	332.	<del>-</del> 5	45.	15.	46.	15.	98.	677.	15.	117.	58.	E
RIRIE		500	533.	500	532.	500.	539.	807.	800.	896.	900	900	900	500.	529.	500.	529.	500.	581.	900	900	773.	798.	798.	803.	500.	518.	500.	517.	500.	533.	500.	400	874.	900	900	900.
WC 19N		57.	48.	48	47.	44.	54.	284.	-	_	က	29.	က	37.	44	31.	44	36.	96	~	537.	189.	42.	16.	21.	29.	33.	~	32.	29.	48.	. 99	578.	490.	144.	59.	
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RIVER DIGITAL MODEL		SYSCON SNAKE	27294.	30455.	32399.	34156.	37048.	38032.	38435.	38257.	38973.	35346.	32920.	33153.	26409.416468.	28913.	30343.	31713.	33356.	36049.	36828.	33777.	28530.	23966.	19104.	15397.	13467.	8376.331441.	16453.	20144.	24176.	27888.	31078.	32349.	32749.	34560.	37819.	33049.	25936.	.10402
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¥-	H. FORK Ashton	9			800.	892	795.	836.	935.	2503	1327		1002	1164.	1169.	13370.	926.	2 4			. 66.	688	747.	691.	928.	518.	1399.	1690.	899.		10760.	734.	521.	667.	461.	4 18.	547.	1126.	2002	1115	1562	1396	614	5	11162.	
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WALCO	EOM	700.	700.	970.	970.	970.	9/0.	970.	970.	970.	952.	952.	. 669	10792.	970.	970.	970.	970.	970.	970.	950.	951.	951.	952.	952.	669	11275.	700.	700.	100	700.	700	970.	970.	970	951.	952	952		. 660	9963.
LAK	INFLOW	3739.	3249.	4031.	3381.	1808.	3772.	7 184.	22826.	19532.	12083.	9938.	6110.	97654.	9930.	4045.	3595.	2835.	1468.	3923.	7042.	7350.	7720.	8859.	8234.	4 168.	69169.	511	490	919	166	146	2307	8865	15787	9313	11162	10550		4086.	64002.
	TFLO	3700.	3124.	3831.	3187.	1651.	3608.			19874.	12131.	9755.	5659.	97391.	9565.	3828.	3377.	2723.	1347.	3946.	7976.	7138.	8216.	9532.	8494.	4294.	70436.	99	312			27.	2319	0000	15722	9401	11697	. 6901		4139.	64322.
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AMEDI	INFLOW	4801.	6865.	6396.	5187.	3651	3608.	7142.	24176.	20242.	8624.	9095.	7369.	81789.107155.167652	5649.	5828.	5377.	4723.	3347.	3946.	3989.	2702.	4491.	7221.	7399.	4258.	58931.	2550	3824		2000	2747	2117	0171		0000	9030	00 00 00 00 00 00 00 00 00 00 00 00 00	. 1010	4990.	72087.
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7002	REXBRG	4537.	2072.	1354.	1264.	1170.	1219.	1974.	10155.	13509.	12289.	9477.	7866.	66887.	3653.	1573.	1041	876.	921.	1637.	3766.	4482.	4331.	6555.	6321.	2417.	37572.	,		. 100	100	. 777	390.	, 83	2209.	7083.	.1766	6267.	5416.	2954.	34395.
	OUTFLO	1668.		355.	259.	235.	258.	584.	1862.	4620.	4550.	2803.	2461.	20141.	640.	277.	263.	247.	219.	327.	1344.	578.	1412.	1731.	1392.	670.	9100.	4		242.	217.		184.	333	1409.	5042	56/2.	6270.	5351.	2784.	28273.
	STANTY	1841	541	369.	265.	236.	259.	600.	2168.		וס ו		2636.	21949.	649.	311.	280.	254.	220.	328.	1384.	890.	1834.	1910.	1515.	873.	10448.		ז פ	- 1	234.	20	80 '	340	•	97	2	6766.	7	66	30311.
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OPCOD JACKSON												12 2												12												12	
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PALISA	2250	7	o c	o c	o c	Ċ	o c	ó	<b>S</b>	<b>S</b>	<b>O</b>	0	3250	ì	· C	C	0	0	0	0	0	0	0	Ö	0300	0000	0077	2250	28	2750	0	0	0	0	0	Ö	0
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AMERICAN FALLS	C	; o	2	9 6	3	<b>.</b>		<i>.</i>		<u>.</u>	<i>i</i> c	Ö				5000	C	Ċ	o.	0	Ó	Ö	c	Ö	•	j (	<b>S</b>	o :	Ö	Ó	Ö	Ó	0	0	Ó	0	0
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LAKE WALCOTT		50.	10		0	<u>.</u>	<u>.</u>		<u>.</u>			ó	00		•									ó		י כ	Ω									0	
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## Table 29.--Column Description, North Side Pumping Division Operation Study

Column	Description
IRRIGATION DEMAND	The irrigation demand is the amount of water, exclusive of precipitation, required for crop production, including losses in application, distribution, and operation of the system. This value represents the combined irrigation demands of the existing Unit A lands (82 percent) and the proposed new Unit A lands (18 percent).
IRRIGATION SHORTAGE	This irrigation shortage equals the irrigation demand minus the water supply (combination of natural flow and storage). The irrigation shortages experienced by the district are voluntary in nature, noting all canal companies in the upper Snake River were assigned voluntary shortages in 1934, 1935, and 1977. These shortages were implemented in an attempt to maximize water use and carryover storage; however, for the A&B Irrigation District, they had sufficient storage in 1934, 1935, and 1977, and had the voluntary shortages not been implemented, they would not have experienced irrigation shortages during the 51-year period examined.
WATER SUPPLY  Natural flow	The order of water use for the operation study was (1) natural flow, (2) American Falls storage, and (3) Palisades storage. The portion of the irrigation supply delivered to the project lands (existing plus new) under the district's
Storage	1939 natural flow right. The portion of the irrigation supply delivered to the project lands (existing plus new) under the district's storage rights.
A&B STORAGE (EOM)	The end-of-month content (EOM) in American Falls and in Palisades Reservoirs

YEAR !	MENTH		ATION		ATION	+	*AW	TER	SUPPL	. Y	+	A&B STOR	AGE (E	(MD
		UEM	IAND	SHOR	IAGE	<b>*</b> *	NATU	RAL	STO	RAGE	+	AMERICAN	PALI	SAD
						+	FLO	w 		rer		FALLS RES		RVO
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE FEET	ACRE	FE
1927	CCT		2230		0		22	_		0		12676		3943
1927	NOV		0		0			0		0		12676		3943
1927	CEC		C		0			0		C		12676		3943
1928	JAN		0		0			0		0		12676		3943 3943
1928	FEB		C		0			0		0		12676		
1928	MAR		0		0			0		0		12676		943
1928	APR		1110		0		11			0		12676		3943 3943
1928	MAY		10070		0		100			0		12676		8943
1928	JUN		13630		0		136		_	0		12676		3208
1928	JUL		18620		0		127	_		913		40913		3208
1928	AUG	3	15400		0			0		400		25513 17913		517
1928	SEP		7600 		0			<u> </u>		600 <del></del> -		11713		
1928	TOTAL		58660		0		397 	47	28	913				
1928	CCT		2050		0		20	50		0		14503		419:
1928	NOV		0		0			C		0		14503		419
1928	DEC		0		0			0		0		14503		419
1929	JAN		0		0			0		0		14503		419
1929	FEB		0		C			0		0		14503		419
1929	MAR		0		0			C		0		14503		419
1929	APR		650		0			50		0		14503		419
1929	MAY		9750		0			50		0		14503		419
1929	NUL		13300		0		133			0		46826		320
1929	JUL		18620		0		31	69		451		31375		320
1929	AUG		15610		0			0		510		15765		320
1929	SEP		708C		0		20	50	ز 	030		10734		464 
1929	TOTAL		67060		0		309	69	36	091				
1929	CCT		1900		C		19	00		0		10100		437
1929	NOV		0		0			0		0		10100		437
1923	DEC		0		0			0		0		10100		437
1930	MAL	•	0		0			0		0		10100		437
1930	FEB		0		0			0		0		10100		437
1930	MAR		0		0			0		0		10100		437
1930	APR		2430		0			30		0		10100		437
1930	MAY		7850		0			50	_	0		46826		320
1930	JUN		12120		0		4 2	26		894		38932		320
1930	JUL		13950		0			0		950		19982		320
1930	AUG	,	13550		0			C		550		6432		320
1930	SEP		6580 		O 					580		0		870
1930	TCTAL	_	63380		0		164	0.6	46	974				

YEAR	MONTH	IRRIGATION		+	WATER	SUPPL	Υ	+	A&B STORA	GE (EDM
		CARMED	SHORTAGE	+	NATURAL	STOR	AGE	+	AMERICAN	PALISA
					FLOW		ER		FALLS RES	RESERV
		ACRE FEET	ACRE FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE F
1930	DCT	1820	0		1820		0		0	781
1930	NOV	0	0		0		0		0	7 9 1
1930	CEC	0	0		0		0		0	731
1931	JAN	C	0		0		0		o o	781
1931	FEB	0	0		0		0		0	781
1931	MAR	0	0		0		0		0	781
1931	APR	1380	0		1380		0		46326	320
1931	MAY	9910	0		7404		06		44320	320
1931	NUL	11860	0		0	118			32460	320
1931	JUL	17340	0		0	173			15120	320
1931	AUG	14320	0		0	143			800	320
1931	SEP	5860	0		0	58 	360		0	5 4 8
1931	TCTAL	62490	0		10604	518	386			
1931	CCT	1600	0		1600	-	0		131	603
1931	VON	0	٥		0		0		131	603
1931	DEC	0	0		0		0		131	603
1932	NAL	C	0		0		0		131	603
1932	FEB	0	0		0		0		131	603
1932	MAR	0	0		C		0		131	603
1932	APR	860	0		860		0		131	603
1932	MAY	7910	0		7910		0		131	603
1932	NUL	10540	. 0		10540		0		131	320
1932	JUL	17930	. 0		3169		761		32065	320
1932	AUG	15740	0		0		740		16325	320
1932	SEP	8760	0				760 		7565	8 9: 
1932	TOTAL	63340	0		24079	39	261			
1932	SCT	2040	0		2040		0		7187	85
1932	NOV	0	C		C		0		7187	85
1932	DEC	0	0		0		0		7187	85
1933	JAN	0	0		0		0		7187	85
1933	FEB	0	. 0		C		0		7187	85
1933	MAR	0	0		0		0		7187	85 85
1933		1530	0		1530		0		7187	85
1933		10070	0		10070		0		7187	8 <i>5</i> 3 <i>2</i>
1933		13230	0		13230		0		46826	
1933		13440	0		C		440		28386	32 32
1933		15740	0		C		740		12545	83
1933	SEP	8190	0		0	۵ 	190 		445ć	
1033	TOTAL	69240	O		26870	42	370			

YEAR	MENTH	IRRIGATION	IRRIGATION	+	WATER	SUPPLY	<b>+</b> 	ARCTZ E3A	GE (EDM)
		DEMAND	SHORTAGE	+	NATURAL	STORAGE	+	AMERICAN	PALISAD
				+	FLOW	WATER	+	FALLS RES	RESERVO
		ACRE FEET	ACRE FEET	+	AC. FT.	AC. FT.	+	ACRE FEET	ACRE FE
1933	CCT	2180	C		2180	0		4784	8937
1933	VGM	0	0		0	0		4784	8937
1933	DEC	0	0		0	0		4784	8937
1934	NAL	0	0		0	0		4784	8937
1934	FEB	0	0		0	0		4784	8937
1934	MAR	0	0		0	0		4784	8937
1934	APR	2340	0		2340	0		42131	3208
1934	MAY	8529	-2558		C	5971		36160	3208
1934	JUN	10139	-3041		0	7098		29062	3208 3208
1934		14799	-4439		C	10360		18702	3209
1934		13479	-4043		C	9436		9266	7047
1934	SEP 	6270	0		0	6270		2996	
1934	TOTAL	57736	-14081		4520	39135			
1934	BCT	2230	0		2230	0		2994	704
1934	NDV	C	0		C	0		2994	704
1934	DEC	C	0		0	0		2994	704
1935	NAL	0	0		. 0	0		2994	704
1935	FEB	O	0		0	0		2994	704
1935		C	0		0	0		2994	704
1935		650	0		65C	0		2994	704
1935		8500	C		9500	0		2994	704
1935		11739	-1126		10613	0		46526	320
1935		17059	-9382		0	7677		39149	320
1935		14810	-8145		0	6665		32485	320
1935	SEP	8020	0		0	8020		24465	713
1935	TOTAL	63008	-18653		21993	22362			
1935	DCT	2040	0		2040	. 0		19879	580
1935	NOV	0	0		C	0		19379	580
1935	DEC	C	0		0	0		19879	580
1936	NAL	0	0		0	0		19879	580
1936	FEE	0	0		0	0		19379	580
1936		0	0		0	0		19879	580
1936		930	0		980	0		19879	580
1936	MAY	10650	0		10650	. 0		19879	580
1936	JUN	12830	C		12830	0		46326	320
1936	JÜL	13540	0		C	18540		28286	320
1936		15450	0		0	15450		12336	320
1936	SEP	7170	0	-	0	7170		5666 	829 
1034	TCTAL	67660	0		26500	41160			

YEAR	MENTH			IRRIG		+	WA.	TER	SUPPL	Y	+	A & B	STOR	AGE (	(EGM)
		DEM	ANU	Shak	AGE		NATU FLC		STOR				ICAN S RES		ISADE SERVOI
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE	FEET	ACI	RE FEE
1936	CCT		2190		0	-	21	90		0			5823		86230
1936	NOV		0		0			0		0			5823		86230
1936	DEC		0		0			0		0			5823		86230
1937	JAN		C		0			0		0			5823		86230
1937	FEB		0		0			0		0			5823		86230
1937	MAR		C		0			0		0			5823		85230
1937	APR		1030		0		10	30		0			5823		86230
1937	MAY		9750		0		97	50		0			5823		86230
1937	JUN	1	1800		0		63	55	54	45			1381		32089
1937	JUL	1	8380		0			0	183	0.8			3001		32089
1937	AUG	1	5250		0			0	152	50			7751		32089
1937	SEP		7300		0			0	73	00			451		83691
1937	TGTAL	6	5700		0		193	25	463	75					
1937	OCT		1990		0		19	90		0			447		82834
1937	NOV		0		0			0		0			447		82884
1937	DEC		0		0			0		0			447		82884
1938	JAN		0		0			0		0			447		82884
1938	FEB		0		0			0		0			447		82884
1938	MAR		C		0			0		0			447		82884
1938	APR		990		0			90		0			447		82884
1938	MAY		9520		С			20		0			447		82334
1938	JUN		2290		0		122			0		_	447		82884
1938	JUL		.6780		C		58	28	109				5374		32089
1938	AUG		5180		0			0	151				0694		32059
1938	SEP		8440		0			0	84 	40			.2254		89771
1938	TOTAL	6	5190		0		306	18	345	572					
1938	DCT		1800		0		18	CO		0			2212		89463
1938	NOV		0		0			0		0			.2212		89463
1938	DEC		0		.0			0		0			2212		89463
1939	JAN		0		0			0		0			2212		89453
1939	FEB		0		0			0		0			2212		89463
1939	MAR		0	,	0			0		0			2212		89463
1939	APR		2270		C		22			0			2212		89463
1939			10600		0		10é			0			6825		32089
1939			12870		0			0		370			33956		32089
1939			18180		0			0		180			15776		32089
1939			15570		0			C		570			20 á		32089
1939	SEP		7620		0			C 		0					78773
1939	TCTAL	6	58910		0		146	570	54	240					

YEAR	MENTH		ATION		ATION	+	WA	TER	SUPPI	_ Y	+	AEB ST	ORA	GE (E	0M) 
		DEM	UNA	SHOR	IAGE	+	NATU	RAL	STE	RAGE	+	AMERICA	N	PALI	SADE
						+	FLC	<b>w</b>	46	TER	+	FALLS R	ES	RESE	RVDI
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE FE	ET	ACRE	FEE
1939	DET		1390		0		13	90		0		16			0800
1939	VCN		0		0			0		0		16			0800
1939	DEC		0		0	•		0		0		16			0800
1940	NAL		0		0			0		0		16			0800
1940	FEB		0		0			0		0		16			0800
1940	MAR		0		0			0		0		16			0800
1940	APR		1270		0		12			0			51		0800
1940	MAY	_	9770		0		31	69		601		4022			2089
1940	JUN		.3250		0			0		250		2697			2089
1940	JUL		.9620		0			0		620		83	_		
1940	AUG		.5530		0		- 0	0		530			0		3963 6573
1940	SEP 		6370				58 	15		555 <del></del>				٠	
1940	TOTAL	6	6200		0		116	44	54 	556 					
1940	DCT		1190		0		. 11	90		0			B <del>9</del>		273
1940	NOV		0		0			0		0			8 9		273
1940	CEC		0		0			C		0			89		3273
1941	JAN		0		0			0		0			89		3273
1941			C		0			C		0			89		3273
1941			0		0			0		0			89		3273
1941			1820		0		18	20	~	0			89		3273 3273
1941			9890		0			0		890		254	89 84		3208
1941			11450		0			0		450 670		78			3203
1941			17670		0			0		110		15	0		2382
1941		4	15110		0		21	78		782			٥		7492
1941	SEP		6960 					. 10							
1941	TOTAL	(	64090		0		61	88	57 	902					
1941			1460		0		14	60		0			0		6985
1941			0		0			0		0			0		6985
1941			0		0			0		0			0		6985
1942			0		0			0		0			0		5985
1942			0		0			0		٥			0		5985 6985
1942			0		0		_	0		0			0		6985 (385
1942			1490		0			490		0			0		6985 4985
1942			8950		0			950		0		2/7	0		6985 2202
1942			11630		0		1	589		041		367			3208
1942			17930		0			0		930		188			3206
1942		;	15280		0			0		280		3 3	75		320S
1942	SEP		7550 		0. 					7550					8 4 6 3 
1942	TETAL	,	64290		0		13,	489	5(	301					

YEAR	MENTH		ATION NAND	IRRIG SHOR	ATION	<b>+</b>	WATER	SUPPL	, Y	+	ARCTZ 83A	GE (EDM)
		D Z M	IAND	3708	INGE	+	NATURAL	STOR	RAGE		AMERICAN	PALISAD
						+	FLOW	WAT	rea	+	FALLS RES	RESERVO
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE FE
1942	GCT		1720		0		1720		0		0	8148
1942	NOV		0		0		0		0		0	8148
1942	DEC		0		0		0		0		0	8148
1943	JAN		0		0		0		0		0	8148
1943	FEB		0		0		0		0		0	3148
1943	MAR		C		0		0		0		0	814
1943	APR		1760		0		1760		0		0	814
1943	MAY		10240		0		10240		0		0	814
1943	JUN		10910		0		10910	_	0		0	814
1943	JUL		18080		0		10589		491		39335	320 320
1943	AUG	]	15620		0		0		<b>620</b>		23715	898
1943	SEP		7890 		0		2113		77 <i>7</i> 		17938	C 7 0
1943	TOTAL		66220		0		37332	28	888 <del></del>			
1943	ECT		1810		0		1810		0		17990	902
1943	NOV		0		0		0		0		17990	902
1943	DEC		0		0		0		0		17990	902
1944	JAN		0		0		. 0		0		17990	902
1944	FEB		0		0		0		0		17990	902
1944			0		C		0		0		17990	902
1944			970		0		970		0		17990	902
1944			8150		0		7404		746		17990	902
1944			9770		0		0		770		17990	902
1944			17910		0		0		910		18400	320
1944			15370		0		0	_	370		3030	320
1944	SEP		8130 				5799 	ک 	331 		699 	897 
1944	TOTAL	(	62110		0		15983	46	127			
1944	DCT		1870		0		1870		0		632	811
1944			C		0		0		0		632	811
1944	DEC		0		0		0		0		632	811
1945			0		0		0		0		632	811
1945			0		0		0		0		632	811
1945			0		0		0		0		632	811
1945			950		0		950		0		632 633	S11
1945			9420		C		9420		0		532	811
1945			11500		0		11500		253		632	811
1945			18080		0		5828		252		34574	320
1945		•	15340		0		0		340		19234	320
1945	SEP		7910 		C 		0		910		11324	907
10/5	TCTAL		65070		0		29568	35	502			

YEAR	MENTH	IRRIGA		IRRIG SHOR	ATION	+	AW 	TER	SUPP	LY	+	AEB	STOR	AGE (E	CMO
		DEMA	IND	2508	inge	+	NATU	RAL	STO	RAGE			ICAN		
						+	FLO	W	WA	TER	+	FALL	S RES	RESE	RVD
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE	FEET	ACRE	FE
1945	DCT	1	1390		0		13	90		0			0515		429
1945	NON		0		0			C		0			0515		429
1945	DEC		0		0			0		0			0515		1429 1429
1946	JAN		0		0			0		0			0515 0515		429
1946	FEB		0		0 0			0		0			0515		429
1946	MAR	9	0 1390		0		13	-		0			0515		3429
1946	APR May		310		0		103			Ö			0515		3429
1946 1946	מטנ		2740		0		127			٥			0515		429
1946	JUL		3030		ů			92	16	438			0388		3208
1946	AUG		5310		Ď			0		310			5078		208
1946	SEP		5740		0		47	66		974			3105	9	9069
1946	TETAL	6.5	5910		0		321	88	33	722					
1946	GCT		1360		0		13	60		0		1	2120		338
1946	VEN		0		0			C		0			2120		338
1946	DEC		0		0			0		0			2120		338
1947	JAN		0		0			0		0			2120		338
1947	FEB		0		0			C		0			2120		338
1947	MAR		0		0			C		0			2120		338
1947	APR		1650		0			50		0			2120		8 2 8
1947	MAY		0800		0		108			0			2120		8 3 8 8 3 8
1947	JUN		1230		. 0		112	67	17	583			9243		320
1947	JUL		8650 5550		0		10	0		550			3693		320
1947 1947	AUG Sep		5550 7130		0		61	.33		997			2596		905
1947	TCTAL	6	6370		0		322	40	34	130					
1947	CCT		1440		0		14	40		0		1	1327		807
1947	VON		0		0			C		0		1	1327		807
1947	DEC		0		. 0			0		0			1327		807
1948	JAN		0		0			O		0			1327		807
1948	FEB		C		0			0		0			11327		207
1948	MAR		0		0			0		0			11327		807
1948	APR		1530		C			30		C			1327		807
1948			0320		0		103			0			11327		807
1948	JUN		2810		C		128		•	0.0			11327		807 320
1948			820C		0		3	710		490			32336 16506		3 2 0 3 2 0
1948 1948			5730 7610		0 ე		5.	0 355		255			15352		905
	TOTAL		 7640		0			 165		475					

YEAR	MENTH			+	WATER	SUPPLY	+	ARS STORA	GE (EDM)
		DEMAND	SHORTAGE	+	NATURAL	STORAGE	+	AMERICAN	PALISADE
				+	FLOW	WATER	+	FALLS RES	RESERVOI
		ACRE FEET	ACRE FEET	+	AC. FT.	AC. FT.	+	ACRE FEET	ACRE FEE
1948	DCT	1170	0		1170	0		13045	76931
1948	NOV	O	0		0	0		13045	76931
1948	DEC	0	0		0	0		13045	76931
1949	MAL	0	Ü		0	0		13045	76931
1949	FEB	C	. 0		0	0		13045	76931 76931
1949	MAR	1650	U		1550	0		13045	75931
1949	APR	1550	0		1550	0		1304 <i>5</i> 13045	76931
1949	MAY	10760	0		10760	0		46826	32089
1949	JUN	12700	. 0		12700			28256	32089
1949	JUL	18570	0		0	18570		12906	32089
1949	AUG	15350	0		0	15350 7310		5596	90421
1949	SEP	7310	0					JJ70	7V=61
1949	TOTAL	67410	0		26180	41230			
1949		1390	0		1390	0		5302	85672
1949		0	0		0	0		5302	85672
1949		0	0		0	0		5302	85677
1950		0	U		0	0		5302	85677 85677
1950		0	0		0	0		5302 5302	8567
1950		1 7 7 0	o o		1220	0		5302	8567
1950		1220 9930	0		1220 9930	0		5302	8567
1950		13250	0		13250	0		5302	8567
1950		18770	0		9522	9248		37578	32089
1950 1950		15470	n		C	15470		22108	3208
1950		8320	ő		Ö	8320		13788	9079
1950	TOTAL	68350	0		35312	33038	-		
1950	CCT	1680	0		1680	0		13776	9071
1950		0	0		0	0		13776	9071
1950		0	0		C	0		13776	9071
1951		0	0		C	0		13776	9071
1951	FEB	C	C		0	0		13776	9071
1951	MAR	0	C		0	0		13776	9071
1951	APR	1540	0		1540	٥		13776	9071
1951	MAY	9650	0		9650	0		13776	9071
1951	NUL	13200	0		13200	0		13776	9071
1951	JUL	18740	0		5286	13454		33372	3208
1951	AUG	14870	C		0	14870		18502	3208
1951	SEP	8490	0		0	8490		10012	9044
1951	TETAL	68170	0		31356	36814			

<u></u>

YEAR	HTNOM		NOITA		ATION TAGE	+	NATER	SUPPL	. Y	+	ARB STORA	GE (ED	M)
		UEM	טאאו	2008	IAGE	+	NATURAL	STOR	RAGE	+	AMERICAN	PALIS	
						+	FLOW	WAT	ER	+	FALLS RES	RESER	{VC
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE	FE
1951	DCT		2020		0		2020		0		9300		+01
1951	NOV		G		0		0		0		9300		401
1951	DEC		0		0		C		0		9300		401
1952	JAN		0		0		0		0		9300		401
1952	FEB		C		0		C		0		9300	-	403
1952	MAR		0		0		0		0		9300		40: 40:
1952	APR	_	1050		0		1050		0		9300 9300		<b>→</b> 0 . <b>4</b> 0 :
1952	MAY		10960		0		10960		. 0		9300		40:
1952	JUN		13940		0		13940	1 2	U 004		32922		20
1952	JUL		19190		0		5286 0		904 210		16712		20
1952	AUG	1	16210		0		0		730		7982		04
1952	SEP		8730		0								
1952	TCTAL		72100		0		33256	38	844 				
1952	SCT		2290		0		2290		0		7415		40
1952	NOV	•	C		0		0		0		7415	8	40
1952	DEC		0		0		0		0		7415		40
1953	JAN		0		0		0		0		7415		40
1953	FEB		0		0		0		0		7415		40
1953	MAR		0		0		0		0		7415		40
1953	APR		1870		0		1870		0		7415		40
1953	MAY	:	10430		0		7404	3	025		7415		40
1953	JUN		12130		0		12130		0		7415		20
1953	JUL		19230		0		0		230		24570		20
1953	ΔÜG		15840		0		0		840		8730		20
1953	SEP		8450 		0		0	8 	450 		280		00
1953	TOTAL		70240		. 0		23694	46	546 				
1953	ECT		1960		0		1960		0		255		17
1953	NOV		. 0		0		0		0		255		17
1953	DEC		0		0		0		0		255 355		17
1954	JAN		0		0		0		0		255 255		17
1954	FEB		0		0		. 0		-0		255		17
1954	MAR		0		0		10/0		0		255		1.7
1954	APR		1940		0		1940 11130		0		255		17
1954	MAY		11130		0		12320		0		255		117
1954	JUN		12320		0 0		4761	1 2	779		33047		20
1954			13540		. 0		4/51 C		610		17437		320
1954 1954			15610 8280		0		0		280		9157		396
	JLF												. — -

YEAR	MONTH		ATION AND	IRRIG Shor	ATION	+	WATER	SUPPL	γ .	AEB STORA	GE (EDM
		JEM	מאאט	31101	INGL	+	NATURAL			+ AMERICAN	
						+	FLOW	WAT	ER .	FALLS RES	RESERV
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+ ACRE FEET	ACRE F
1954	CCT		1640		0		1640		0	8544	836
1954	NDV		C		0		0		0	8544	836 836
1954	DEC		0		0		0		0	8544	836
1955	JAN		0		0		0		<b>0</b> 0	8544 8544	836
1955	FEB		0		0		0		0	8544	836
1955	MAR		0		0		7.0		0	8544	836
1955	APR		720		0		720		0	8544	836
1955	MAY		9550		0		9550	1 2	•	45620	320
1955	JUN		13360		0 0		12154 0	12 185		27040	320
1955	JUL		8580		0		0	159		11130	320
1955	AUG	4	15910		٥		1589	70		4119	898
1955 	SEP		8600							7 ° ° ′	
1955	TGTAL		836C		0		25653 	427 	07		
1955	CCT		1490		0		1490		C	3799	819 819
1955	NOV		0		0		0		0	3799 3799	819
1955	DEC		0		0		0		0	3799	819
1956	JAN		0		0		0		0	3799	819
1956	FEB		0		0		0		0	3799	81
1956	MAR		0		0		1870		Õ	3799	81
1956	APR		1870 10570		0		10570		٥	3799	81
1956	MAY		13010		0		13010		٥	46826	81
1956	JUN		19020		Ö		0	190	•	27306	32
1956	JUL		15020		Ö		Č	150		12786	32
1956 1956			7820		Ö		1064		756	6030	89
1956	TGTAL		68800		0		28004	407	796		
1956	GCT		1460		0		1460		0	5359	79
1956	VCN	•	0		0		0		0	5359	79
1956	DEC		0		0		0		0	5359	79
1957			0		0		0		0	5359	79
1957			C		0		C		0	5359	79 79
1957			. 0		C		0		0	5359 5359	79
1957			1100		0		1100		0	5359	79
1957			9820		0		9820		0	5359	79
1957			12930		0		12930 4236	4.7	334	31992	32
1957			19070		0		4236		360	16632	32
1957 1957			15360 7970		0		0		970	8562	90
	TOTAL		67710				29546		 164		

YEAR	MENTH	IRRIGATION DEMAND	IRRIGATION SHORTAGE	+	WATER	SUPPLY	+ -+-	ARB STORA	GE (EDM)
		02112110	J. J. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.					AMERICAN	
				+	FLOW	WATER	+	FALLS RES	RESERVE
		ACRE FEET	ACRE FEET	+	AC. FT.	AC. FT.	+	ACRE FEET	ACRE F
1957	DCT	1390	0		1390	0		8114	845
1957	NOV	0	0		0	0		8114	8 4 5 3
1957	DEC	0	0		0	0		8114	845
1958	JAN	0	0		0	C		8114	845
1958	FEB	0	0		0	0		8114	845
1958	MAR	0	0		0	0		8114	845
1958	APR	1530	0		1530	0		8114	845
1958	MAY	10650	0		10650	0		8114	845
1958	JUN	12940	0		6355	6585		40241	320
1958	JUL	19040	0		0	19040		21201	320
1958	AUG	15180	0		0	15180		6021	320 839
1958	SEP	7280	0		1589	5691 		330	037
1958	TOTAL	68010	.0		21514	46496			
1958	CCT	2010	0		2010	.0		317	853
1958	VOV	0	0		C	0		317	853
1958	DEC	0	0		0	0		317	853
1959	JAN	0	0		0	0		317	853
1959	FEB	0	0		C	0		317	853
1959	MAR	0	. 0		0	0		317	853
1959	APR	2130	0		2130	0		46826	320
1959		9450	0		9450	0		46826	320
1959	JUN	12500	0		12500	0		46826	320
1959		18770	C		4236	14534		32292	320
1959		15020	0		C	15020		17272	320
1959	SEP	6470	0		3178	3292		13979	884 
1959	TOTAL	66350	0		33504	32846			
1959	ECT	970	0		970	0		11348	749
1959	VEN	0	0		0	0		11848	749
1959	DEC	0	0		0	0		11848	749
1960	JAN	0	0		0	0		11848	7 4 9
1960	FEB	0	0		0	0		11348	749
1960	MAR	0	0		C	0		11848	749
1960	APR	1710	. 0		1710	0		46826	320
1960	MAY	10590	0		O	10590		36236	320
1960		13230	0		Ō	13230		23006	320
1960		19230	0		0	19230		3776	320
1960		15150	0		0	15150		0	192
1960	SEP	8070	0		0	8070		0	514
1940	TETAL	68950	0		2680	56270			

YEAR	MONTH		ATION			+	WATER	SUPPL	Υ	+	A&B STORA	GE (EDM)
		UEM	DMAI	SHOR	IASE	+-	NATURAL	STOR	AGE	+	AMERICAN	PALISA
							FLOW		ER		FALLS RES	RESERV
		ACRE	FEET	ACRE	FEET	+	AC. FT.	AC.	FT.	+	ACRE FEET	ACRE F
1960	DCT		1800		0		1800		0		0	413
1960	NOV		0		٥		0		0		0	413
1960	DEC		0		0		0		0		0	413
1961	JAN		0		0		O		0		0	413
1961	FEB		0		0		0		0		0	413
1961	MAR		0		0		0		0		0	413
1961	APR		1940		0		1940		0		45687	320
1961	MAY		9910		0		0		10		35777	320
1961	אטנ		2500		0		0	125			23277	320
1961	JUL		9250		0		0	192			4027	320
1961	AUG	1	4420		0		0	144	_		0	203
1961	SEP		5590		0	-	5590 		0			63
1961	TOTAL		55410		0		9330	560	080			
1961	BCT		1500		0		1500		0		632	154
1961	VON		0		0		0		0		632	164
1961	DEC		0		0		C		0		632	164
1962	NAL		0		0		0		0		632	164
1962	FEB		C		0		C		0		632	164
1962	MAR		0		0		С		0		632	164
1962	APR		1770		0		1770		0		632	164
1962	MAY		9990		0		9990		C		632	164
1962	JUN	1	12600		0		12600		. 0		46826	320
1962	JUL	1	19640		0		3694		946		30880	320
1962	AUG	1	15720		0		. C		720		15160	320
1962	SEP		9140		0		0	9:	140		6020	899
1962	TOTAL		70360		0		29554	40	806		~~~~	
1962	DCT		1990		0		1990		Ō		5443	813
1962	NOV		0		0		0		Ó		5443	813
1962	DEC		0		0		0		0		5443	813
1963	JAN		0		0		0		0		5443	813
1963	FEB		0		0		0		0		5443	813
1963	MAR		0		0		0		0		5443	813
1963	APR		1380		0		1380		0		5443 = ( / 3	813
1963	MAY		9830		0		9830		0		5443	813
1963	JUN		12270		0		12270	4.4	0		46826	813
1963	JUL		20100		0		1067		033		27793	320
1963	AUG		15510		0		1500		510		12283	320 904
1963	SEP		3180 		0 		1589	6 	591 		5592 	7U4
1963	TOTAL		69250		0		23126	41	134			

YEAR	MONTH	IRRIGATION DEMAND		IRRIGATIO SHORTAGE		*	WA	TER	SUPP	LY	+	E3A	STORA	GE (E	(MU 
		UEM	IANU	2478	IAGE	+	NATU	RAL	STC	RAGE	+	AMER	ICAN	PALI	SAD
						+	FLE	W	AW	TER	+	FALL:	S RES	RESE	2 V O
9 and 400 and		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE	FEET	ACRE	FE
1963	CCT		2700		0		27	00		0			5078		067
1963	NOV		0		0			0		0			5078		067
1963	DEC		0		0			0	•	0			5078	_	067
1964	JAN		0		0			0		0			5078		063
1964	FEB		0		0			0		0			5078		06
1964	MAR		0		0		_	0		0			5078		06
1964	APR	_	720		0			20		0			5078		06
1964	MAY		0080		0		100			0			5078		06
1964	JUN		2190		0		121		• •	0			5078		06
1964	JUL		20190		0		13	46		244			4582		20
1964	AUG	3	16440		0			0		440			8142 6153		97
1964	SEP		8990		0					990			9152 		
1964	TOTAL		71310		0		336	36	37 	674					
1964	DCT		2620		0		2	520		0			8283		12
1964	VON		0		0			C		0			8283		12
1964	DEC		0		0			0		0			8283		112
1965	JAN		0		0			0		0			8283		312 312
1965	FEB		0		U			0		0			8283 8283		312
1965	MAR		0		Ü		٠.	•		0			8283		312
1965	APR		1770		0			770 680		0			8283		312
1965	MAY		10680		0			370		0			8283		812
1965	JUN		13370		C			303	1.4	357			2469		320
1965	JUL		19660				٠,	0		5450			7019		320
1965 1965	AUG Sep	•	15450 8060		0			Ö		3060		•	8959		904
							22	 743		 7867					
	TCTAL		71610												
1965	CCT		3010		0		. 3	010		0	,		8656		873 873
1965	NOV		0		0			0		0			8556		873
1965	CEC		0		0			0		0			8656		873 873
1966	JAN		0		0			0		0			8656 8656		873
1966	FE8		0		0			0		C			6826		320
1966	MAR		0		0		,	0		0			6826		32(
1965	APR		2530		C		2	530		0					320
1966	MAY		11240		0			0		1240			3586 3086		320 320
1966	JUN		12500		0			0		2500		2	4005		320 320
1966	JUL		19080		0			0		9080			4005		320 191
1966	AUG		15460		0			0		5460 8320			o o		2 7 1 5 7 1
1966	SEP		8320 		0 										
1966	TCTAL		72140		0		5	540	5.	5600					

YEAR MONTH	MENTH	IRRIGATION CEMANO			IRRIGATION SHORTAGE		WATER		SUPPLY		+ -+-	AEB STOR	AGE (E	(MC
	UEF	TANU	SPUKTAGE		+	NATURAL					AMERICAN		PALISADE RESERVOI	
					~~~~	+	FLO	W 	A W	TER		FALLS RES	, KEJE	. K V U 1
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE FEET	ACRE	FEE
1966	BCT		2670		0			0	2	670		0		4029
1966	NOV		0		0			0		0		0		54029
1966	CEC		0		0			C		0		0		54021
1967	JAN		0		0			0		0		0		5402
1967	FEB		0		0			0		0		0		5402
1967	MAR		C		0			C		0		0		5402
1967	APR		1820		0		18			0		0		5402
1967	MAY		10270		0		102			0		0		5402
1967	MUL	1	12560		0		125			0		0		5402
1967	JUL	7	20030		0		58	28		202		32624		3208
1967	AUG	1	16540		0			0		540		16084		3208
1967	SEP		9430		0			0	9 	430		6654		8865 
1967	TETAL		73320		0		304	78	42	342 				
1967	DCT		2540		0		25	40		0		6354		8465
1967	VON		0		0			C		Đ		6354		8465
1967	DEC		0		Q			0		0		6354		8465
1968	JAN		0		0			0		0		6354		8465
1968	FEB		0		0			0		0		6354		8465
1968	MAR		C		0			. 0		0		6354		8465
1968	APR		1920		0		19	20		0		6354		8465
1968	MAY		11220		C		112	20		0		6354		8465
1968	JUN		13410		0			0	13	410		33416		3209
1963	JUL	;	20190		0			0	20	190		13225		3203
1968	AUG	•	13610		0			0		610		0		3165
1968			7930		0			0	7	930		0 		787 <i>6</i> 
1968	TOTAL		70820		0		156	80	55	140				
1968	DCT		2350		0		23	50		0		0		7368
1968	VCN		0		0			0		0		0		7868
1968	DEC		0		0			0		0		0		7868
1969	JAN		0		Ö	•		0		0		0		7368
1969	FEB		0		C			C		0		0		736
1969	MAR		0		C			0		0		o		7868
1963			2260		0			50		0		0		7368
1969			11460		0		5 8	28		532		0		7868
1969			12940		0			0		940		0		786
1969			19230		0			0		230		9024		320
1969			16070		O			0		070		្រ		2411
1969			7930		0			0	7	930		0		2983
	TETAL		72240		0		104	38	41	802				

YEAR	MONTH	IRRIGATION	IRRIGATION	+ WATER		SUPPLY	+	AEB STORA	GE (EDM)
		DEMAND	SHORTAGE		NATURAL			AMERICAN FALLS RES	PALISADE RESERVOI
				+ 	FLOW	WATER	<b>+</b>	LATES KES	
		ACRE FEET	ACRE FEET	+	AC. FT.	AC. FT.	+ 	ACRE FEET	ACRE FEE
1969	CCT	1790	0		1790	0		2386	90800
1969	NOV	0	0		0	. 0		2886 2886	90800 90800
1969	DEC	0	0		U	U		2886	90800
1970	JAN	. 0	0		0	0		2886	90800
1970	FEB	0	0		0	0		2886	90800
1970	MAR		0		1440	0		2886	90800
1970	APR	1440 9910	0		9910	0		2886	90800
1970	MAY	12770	0		12770	. 0		2886	90800
1970 1970	JUL	19340	0		12110 C	19340		27486	32039
1970	AUG	15280	0		Ğ	16280		11206	32089
1970	SEP	7290	ő		6657	633		10573	76369
1970	TOTAL	68820	0		32567	36253	-		
1970	CCT	2030	0		2030	0		14073	90800
1970	NDV	0	0		0	0		14073	90800
1970	DEC	0	0		0	. 0		14073	90800
1971	MAL	0	0		0	0		14073	90800
1971	FEB	, Ο	0		0	0		14073	90800
1971	MAR	0	0		0	0		14073	90800
1971	APR	1110	0		1110	0		14073	90800
1971	MAY	9490	0		9490	0		14073	90800
1971		12900	0		12900	C		14073	9080C
1971		20320	0		7946	12374		34452	32089
1971		16780	0		0	16780		17672	32081
1971	SEP	8290	0		0 	8290		9382	9023
1971	TCTAL	70920	0		33476	37444			
1971		1900	0		1900	0		11066	9080
1971		0	0		0	0		11065	9080
1971		0	0		0	0		11066	9080
1972		0	0		0	0		11066 11066	9080
1972		0	0		0 <b>0</b>	0		11066	9080
1972		1 = ( 0	0		1540	0		11066	9080
1972		1540	0		10740	0		11065	9080
1972		10740	. 0		12940	0		11065	9080
1972		12940 20000	0		3710	16290		30536	3208
1972		15750	0		0	15750		14786	3208
1972 1972		7600	Ö		2653	4947		9840	9011
	TCTAL	70470	0		33483	36987			

TEAR	MENTH	IRRIGATION DEMAND			IRRIGATION SHORTAGE		WA	TER	SUPPLY		<b>+</b>	AEB STORAGE (			(MD
				SHOR			NATU						ICAN		
						+	FLO	W 	'AW	TER	+	FALL	S RES	RES	ERVE
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE	FEET	ACR	E F
1972	BCT		1660		0		16	60		0			2047		9080
1972	NOV		0		0			0		0			2047		9080
1972	DEC		0		0			0		0			2047		9080 9080
1973	JAN		0		0			0		0			2047 2047		908(
1973	FEB		0		0			0					2047		908
1973	MAR		0		0		4.6	0		0			2047		908(
1973	APR	_	1090		0			90	,	0			2600		908
1973	MAY		10580		0		53	54		226			9640		320
1973	JUN		12960		0			0		960			0300		320
1973	JUL		19340		0			0		340		1	0300		264°
1973	AUG	3	15260		0		2 1	0 78.		260 382			0		573:
1973	SEP		7560 		0										
1973	TOTAL		68450 		0		122	282	56 	168					
1973	CCT		1830		0		18	30		0			2002		908
1973	NOV		0	•	0			O		0			2002		908
1973	DEC		C		0			0		0			2002		908
1974	JAN		0		0			0		0			2002		908
1974	FEB		C		0			0		0			2002		908
1974			0		0			0		0			2002		908
1974			1140		0			140		0			2002		908
1974			10870		0		10			0			2002		908
1974			13370		0			370	• •	0		-	2002		320
1974			19290		0		<b>)</b>	500		790			17796		320
1974			15240		,0 0			0		240			9996		718
1974	SEP		7800 		·								,,,,,		
1974	TCTAL		69540 		0		3 2	710	3 <i>6</i>	830					
1974	DCT		2040		0		2	040		0			5353		908
1974	NON		0		0			0		0			5358		908
1974	DEC		0		0			0		0			5358		908
1975			0		0			0		0			5358		908
1975			C		0			0		0			5358		908
1975			0		0			0		0			15358		908
1975			510		0			510		0			15358		908
1975			5200		0			200		0			15358		908
1975			12760		0			760		0			15358		320
1975			20270		0		12	182		3088			38738 22343		320
1975			16390		0			0		3920			13428		848
1975	SEP		8920 												
	TOTAL		66090		C		2.2	692	2 7	3398					

YEAR MONTH	MCNTH	IRRIGATION DEMAND				+	WA.	TER	SUPP	LY	*	SACTS 834	AGE (E	OM)
		LEM	LND	· SHORTAGE		+	NATU	RAL	STC	RAGE		AMERICAN	PALI	
						+	FLD	W 	AW	TER	+	FALLS RES	RESE	2007
		ACRE	FEET	ACRE	FEET	+	AC.	FT.	AC.	FT.	+	ACRE FEET	ACRE	FEE
1975	CCT		1980		0		19			0		12236		727 €
1975	VON		0		0			0		0		12236		727 €
1975	DEC		0		0			0		0		12236		727
1976	JAN		0		0			0		. 0		12236		727 4
1976	FEB		0		0			C		0		12236		727 4
1976	MAR		0		0			0		Ü		12236		727 <
1976	APR	_	540		0			40	·	0		12236		727
1976	MAY		0380		0		103			0		12236		727
1976	JUN		3970		0		139		47	707		46825		208
1976	JUL		20430		C		26			787		29039		208
1976	AUG	3	15070		0		,	0		070		13969 11330		895
1976	SEP		7390 		0			50	۷ 	640 		11330		
1976	TOTAL		9760		0		342	63	35 	497 				
1976	CCT	1750		0			17	50	0			12695		080
1976			0		0			0		0		12695		080
1976	DEC		0		0			0		0		12695		080
1977	JAN		0		0			0		0		12695		080
1977			0		0			0		0		12695		080
1977			0		0		2.5	0		0		12695		080
1977			2510		0		25	10		2 ( 2		46826		208
1977			9059		-2717			0		342		40484 32371		208
1977			11589		-3476			C		113		20737		208
1977			16619		-4935			0		634 331		11406		208
1977 1977		4	13329 5490	-	8 <del>6</del> 28 - 0		31	.62		328		9078		208
1977	TOTAL		50346	- ;	L5176		74	22	37	748				
1977	GCT		1030		0		10	30		0		5759		035
1977			0		O			C		0		5759	. 2	035
1977			0		0			0		0		5759	2	035
1978			0		0			0		0		5759	2	035
1978			O		0			0		0		5759		035
1973	MAR		0		0			. 0		0		575 <del>9</del>		035
1978	APR		720		0			20		0		575 <del>9</del>		035
1978	MAY		9640		0			43	é	397		5759		035
1978	אטנ		12810		0		128			0		5759		035
1978			18480		0		47		13	719		26110		208
1973			15120		0		151			0		26110		203
1978	SEP		5280		0		52	280		0		26110		005
1978	TCTAL		63080		C		423	364	20	716				